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8/27/2025 \\rsandh.com\Files\Projects\NCDOT\10034734006_Polk_112\03.00 Project Execution\03.04 Design\Structures\CAD\400_001_DF183142075090_SMJ_TSH_000_740112.dgn

09/08/2019

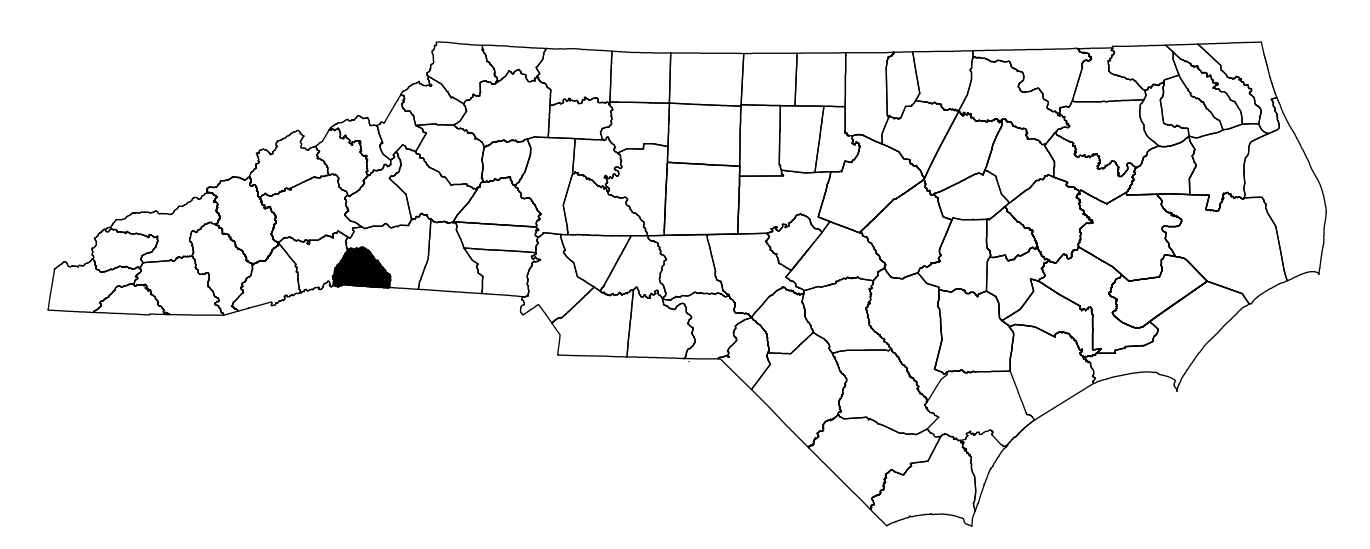
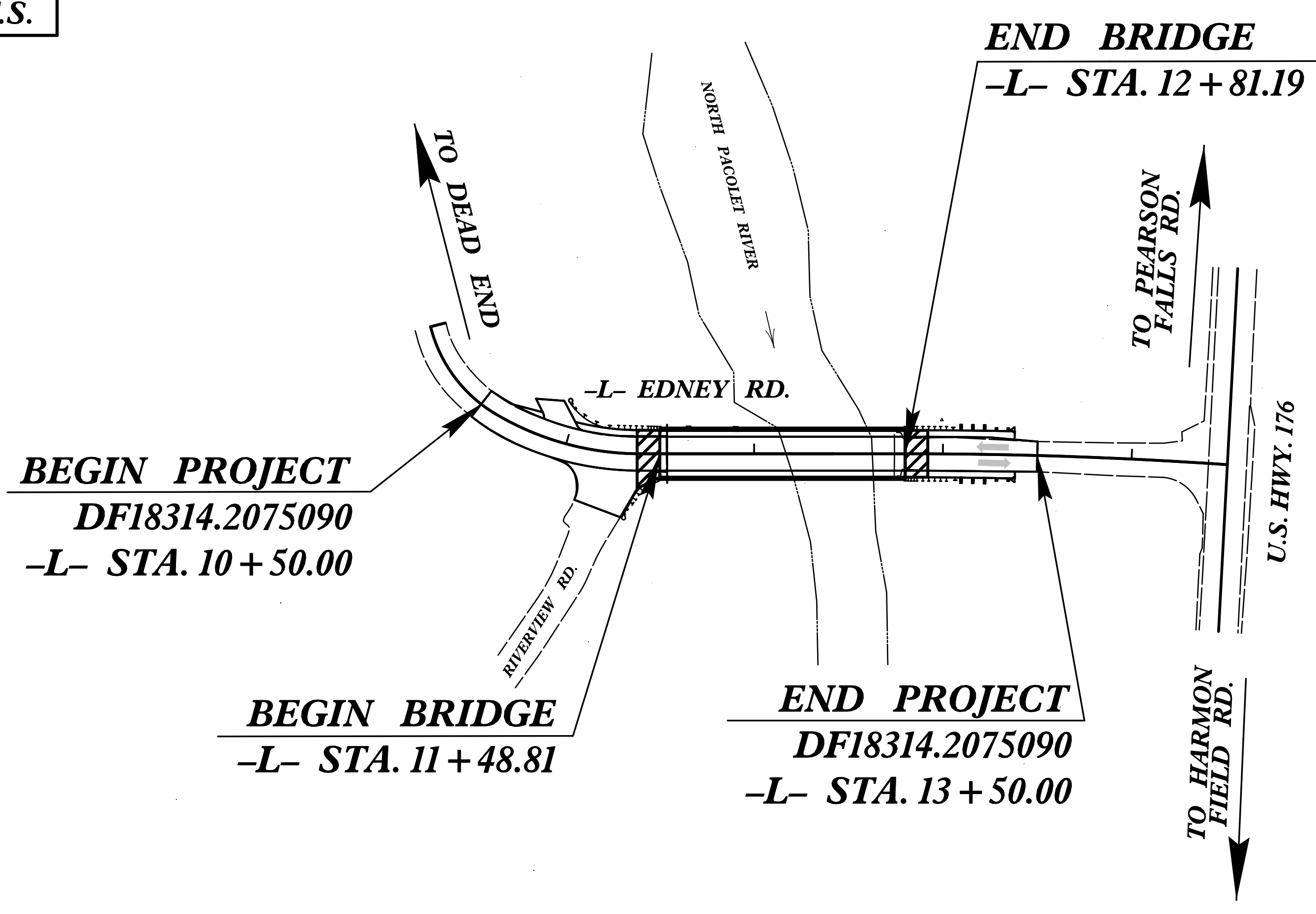
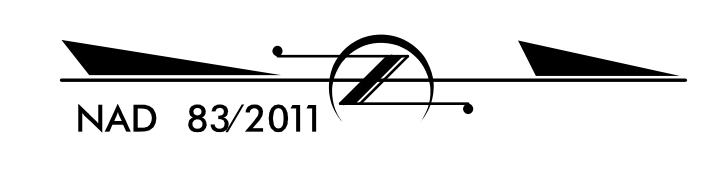
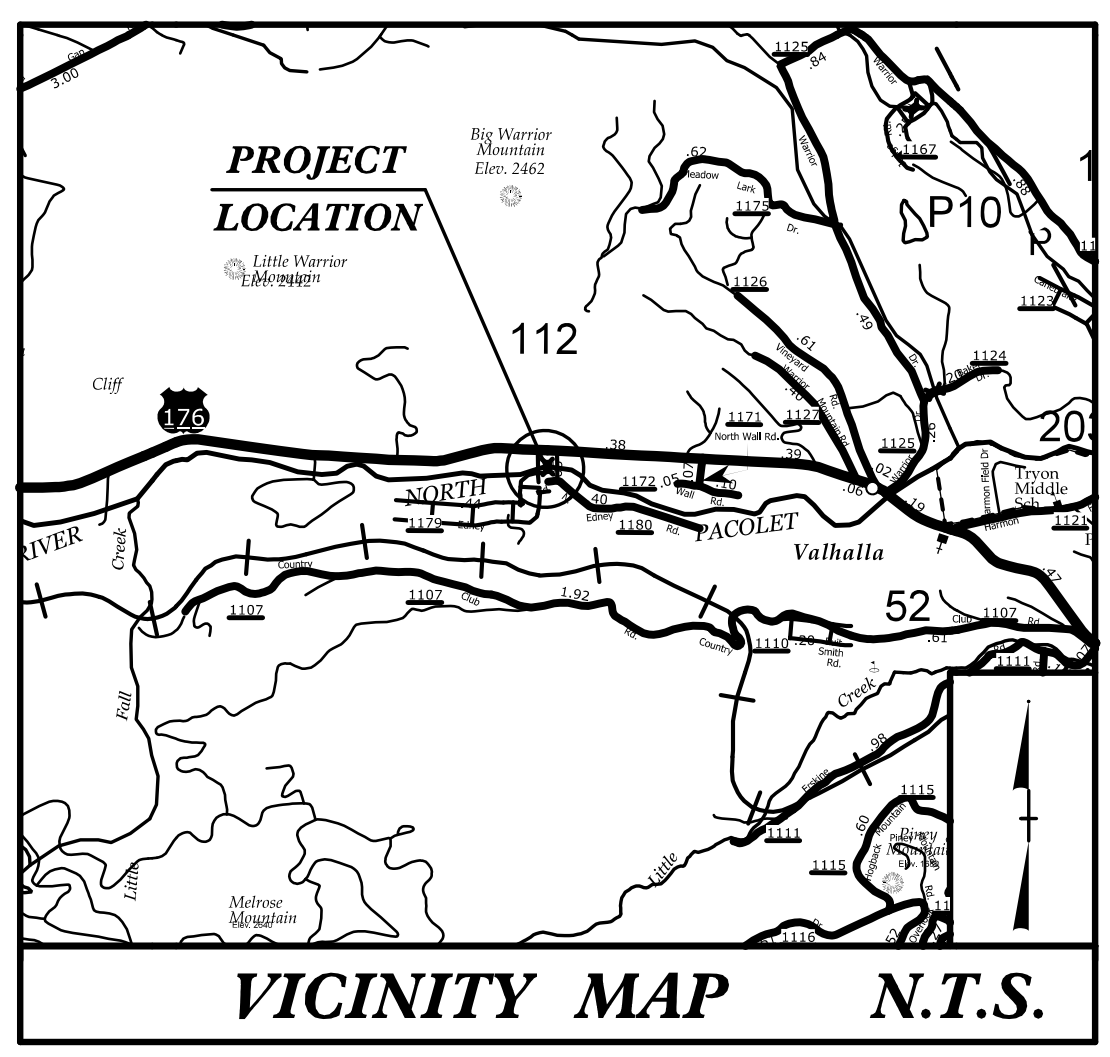
CONTRACT: DN01119 **PROJECT TIP: DF18314.2075090**

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

POLK COUNTY

**LOCATION: BRIDGE 740112 OVER PACOLET RIVER
ON EDNEY ROAD**
TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	DF18314.2075090	1	
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
DF18314.2075090	NA	PE	



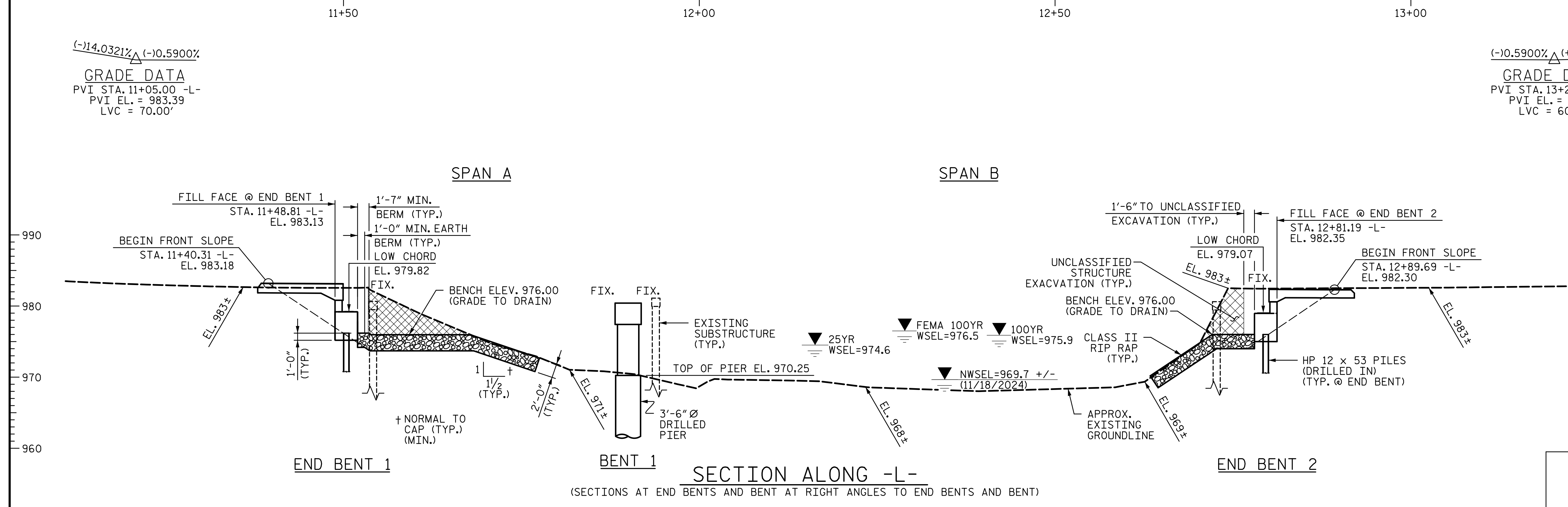
STRUCTURE

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

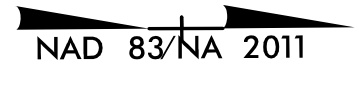
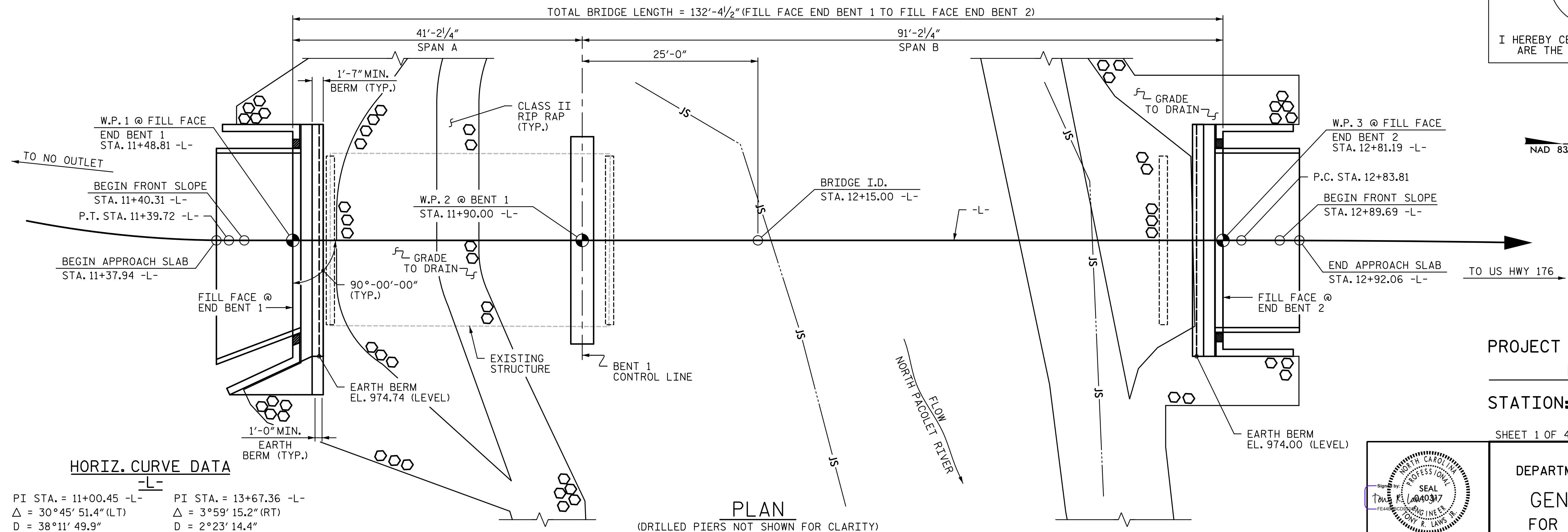
<p>DESIGN DATA</p> <p>ADT 2025 = < 400 ADT 2045 = < 400</p> <p>V = 35 MPH</p> <p>FUNC CLASS = LOCAL SUBREGIONAL TIER</p>	<p>PROJECT LENGTH</p> <p>LENGTH ROADWAY PROJECT DF18314.2075090 = 0.032 MILES LENGTH STRUCTURE PROJECT DF18314.2075090 = 0.025 MILES TOTAL LENGTH PROJECT DF18314.2075090 = 0.057 MILES</p>	<p>PREPARED IN THE OFFICE OF: RS&H 1520 SOUTH BOULEVARD, SUITE 200 CHARLOTTE, NC 28203 NC FIRM LICENSE No: F-0493</p> <p>FOR THE NORTH CAROLINA DEPARTMENT OF TRANSPORTATION 2024 STANDARD SPECIFICATIONS</p> <p>RIGHT OF WAY DATE: MARCH, 2025</p> <p>LETTING DATE: OCTOBER, 2025</p>	<p>STRUCTURAL ENGINEER</p> <p>TONY R. LAWS JR., PE PROJECT ENGINEER</p> <p>MATTHEW R. ACOSTA, PE PROJECT DESIGN ENGINEER</p> <p>ZACHARY T. SHULER, PE NCDOT CONTACT</p>	<p>Seal of the Professional Engineer, Tony R. Laws Jr., License No. 040317.</p> <p>Signed by: <i>Tony R. Laws Jr.</i> SIGNATURE: _____ P.E.</p> <p>10/7/2025</p>	
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GRADE DATA
 (-)14.0321% (-)0.5900%
 PVI STA. 11+05.00 -L-
 PVI EL. = 983.39
 LVC = 70.00'

GRADE DATA
 (-)0.5900% (+)3.4033%
 PVI STA. 13+20.00 -L-
 PVI EL. = 982.12
 LVC = 60.00'

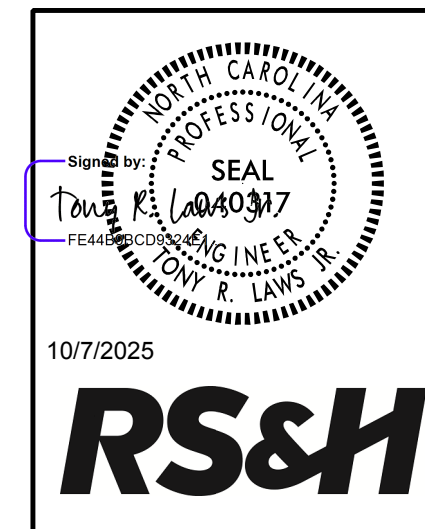


I HEREBY CERTIFY THESE PLANS ARE THE AS-BUILT PLANS.



HORIZ. CURVE DATA
 -L-
 PI STA. = 11+00.45 -L- PI STA. = 13+67.36 -L-
 Δ = 30°45' 51.4" (LT) Δ = 3°59' 15.2" (RT)
 D = 38°11' 49.9" D = 2°23' 14.4"
 L = 80.54' L = 167.03'
 T = 41.27' T = 83.55'
 R = 150.00' R = 2,400.00'

PROJECT NO. DF18314.2075090
 POLK COUNTY
 STATION: 12+15.00 -L-
 SHEET 1 OF 4 REPLACES BRIDGE NO. 740112

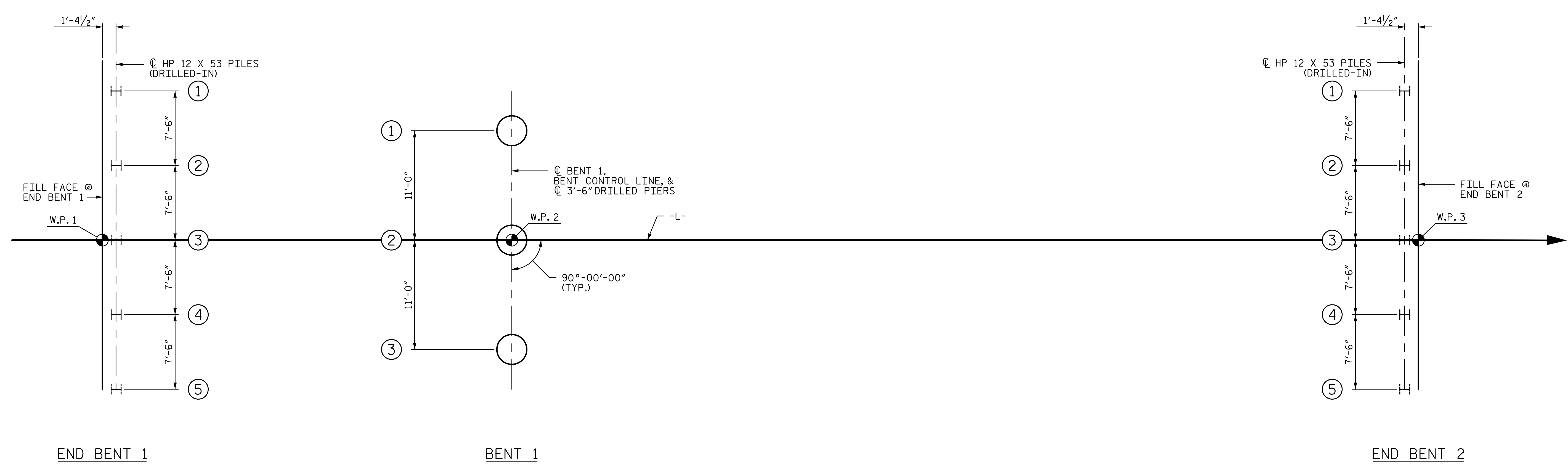
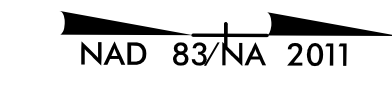


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
GENERAL DRAWING
 FOR BRIDGE ON SR 1179
 (EDNEY RD.) OVER
 NORTH PACOLET RIVER
 BETWEEN SR 1180
 AND US HWY 176

DRAWN BY : T. R. LAWS DATE : 02/2025
 CHECKED BY : M. KHIN DATE : 02/2025
 DESIGN ENGINEER OF RECORD: T. R. LAWS DATE : 10/2025

DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-1
1			3			TOTAL SHEETS
2			4			24



FOUNDATION LAYOUT

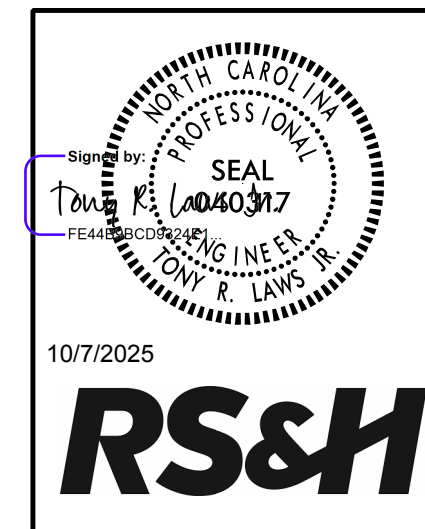
(DIMENSIONS SHOWN ARE TO THE C OF THE PILES OR DRILLED PIERS)

FOUNDATION NOTES:

- FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.
- POLYMER SLURRY CONSTRUCTION IS REQUIRED FOR THE CONSTRUCTION OF BENT NO. 1.
- FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- FILL HOLES FOR PILE EXCAVATION AT END BENT NO. 1 AND END BENT NO. 2 WITH CONCRETE.
- BEFORE FILLING HOLES FOR PILE EXCAVATION AT END BENT NO. 1 AND END BENT NO. 2, DRIVE PILES TO THE REQUIRED DRIVING RESISTANCE.
- PILE EXCAVATIONS ARE RECOMMENDED AT END BENT NO. 1 AND END BENT NO. 2 TO MINIMUM DEPTHS OF 10 FT. TO REDUCE EARTH-BORNE VIBRATIONS ON ADJACENT STRUCTURES. REFER TO SPECIAL PROVISIONS FOR INSPECTION AND MONITORING REQUIREMENTS.
- ONE BORING WAS PERFORMED AT THE INTERIOR BENT AND WEATHERED ROCK WAS ENCOUNTERED AT ELEV. 926.2 FT. THIS ELEVATION IS ASSUMED FOR ALL THREE PIERS. IN ORDER TO SATISFY THE 8 FT ROCK SOCKET REQUIREMENT, THE TIP ELEVATIONS MAY NEED TO BE EXTENDED AT SOME OR ALL PIER LOCATIONS.

PROJECT NO. DF18314.2075090
POLK COUNTY
 STATION: 12+15.00 -L-

SHEET 2 OF 4



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
GENERAL DRAWING
 FOUNDATION LAYOUT

DRAWN BY :	J. SCACCA	DATE :	02/2025
CHECKED BY :	T. R. LAWS	DATE :	02/2025
DESIGN ENGINEER OF RECORD:	T. R. LAWS	DATE :	10/2025

DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-2
1			3			TOTAL SHEETS
2			4			24

SUMMARY OF PILE INFORMATION/INSTALLATION

(Blank entries indicate item is not applicable to structure)

End Bent / Bent No, Pile(s) #(-#) (e.g., "Bent 1, Piles 1-5")	Number of Piles per Line	Factored Resistance per Pile KIPS	Pile Cut-Off (Top of Pile) Elevation FT	Estimated Pile Length per Pile FT	Scour Critical Elevation FT	Driven Piles			Predrilling for Piles **			Drilled-In Piles		
						Minimum Pile Tip (Tip No Higher Than) Elevation FT	Required Driving Resistance (RDR)* per pile KIPS	Pile Redrives Quantity EACH	Predrilling Length per Pile LIN FT	Predrilling Elevation (Elevation Not To Predrill Below) FT	Maximum Predrilling Diameter INCHES	Pile Excavation (Bottom of Hole) Elevation FT	Pile Excavation Not In Soil per Pile LIN FT	Pile Excavation In Soil per Pile LIN FT
End Bent 1, Piles 1-5	5	238	977	45			397					967		10
End Bent 2, Piles 1-5	5	238	976	45			397					966		10
TOTAL QUANTITY:														100

* $RDR = \frac{\text{Factored Resistance} + \text{Factored Drag Load} + \text{Factored Dead Load}}{\text{Dynamic Resistance Factor}} + \text{Nominal Drag Load Resistance} + \text{Nominal Resistance from Scourable Material}$

** Predrilling for Piles is required for end bents/bents with a predrilling length and at the Contractor's option for end bents/bents with predrilling information but no predrilling length.

PILE DESIGN INFORMATION

(Blank entries indicate item is not applicable to structure)

End Bent / Bent No, Pile(s) #(-#) (e.g., "Bent 1, Piles 1-5")	Factored Axial Load per Pile KIPS	Factored Drag Load per Pile KIPS	Factored Dead Load * per Pile KIPS	Dynamic Resistance Factor	Nominal Drag Resistance per Pile KIPS	Nominal Scour Resistance per Pile KIPS
End Bent 1, Piles 1-5	238			0.6		
End Bent 2, Piles 1-5	238			0.6		

* Factored Dead Load is factored weight of pile above the ground line.

SUMMARY OF PILE ACCESSORIES

(Blank entries indicate item is not applicable to structure)

End Bent / Bent No, Pile(s) #(-#) (e.g., "Bent 1, Piles 1-5")	Pipe Pile Plates EACH	Steel Pile Points		
		Pipe Pile Cutting Shoes EACH	Pipe Pile Conical Points EACH	H-Pile Points EACH
End Bent 1, Piles 1-5				5
End Bent 2, Piles 1-5				5
TOTAL QUANTITY:				10

SUMMARY OF DRILLED PIER INFORMATION/INSTALLATION

(Blank entries indicate item is not applicable to structure)

End Bent / Bent No, Pier(s) #(-#) (e.g., "Bent 1, Piers 1-3")	Number of Piers per Line	Factored Resistance per Pier KIPS	Required Drilled Pier Tip Elevation FT	Required Tip Resistance per Pier KSF	Scour Critical Elevation FT	Minimum Drilled Pier Penetration Into Weathered Rock or Rock per Pier LIN FT	Drilled Pier Length* per Pier LIN FT	Drilled Pier Length Not in Soil* per Pier LIN FT	Drilled Pier Length In Soil* per Pier LIN FT	Permanent Steel Casing Required? YES	Permanent Steel Casing Tip Elevation (Elevation Not To Extend Casing Below) FT	Permanent Steel Casing Length** per Pier LIN FT
Bent 1, Piers 1-3	3	685	918	20	956	8	53			Yes	958	13
TOTAL QUANTITY:							159					39

* Drilled Pier Length, Drilled Pier Length Not in Soil and Drilled Pier Length in Soil represent estimated drilled pier quantities and are measured and paid for as either "___ Dia. Drilled Piers" or "___ Dia. Drilled Piers Not in Soil" and "___ Dia. Drilled Piers in Soil" in accordance with Article 411-7 of the NCDOT Standard Specifications. For bents with a not in soil pay item, drilled piers through air or water will be paid at the contract unit price for "___ Dia. Drilled Piers in Soil."

** Permanent Steel Casing Length equals the difference between the ground line or top of drilled pier elevation, whichever is higher, and the permanent casing tip elevation and is measured and paid for as "Permanent Steel Casing for ___ Dia. Drilled Pier" in accordance with Article 411-7 of the NCDOT Standard Specifications.

SUMMARY OF DRILLED PIER TESTING

(Blank entries indicate item is not applicable to structure)

End Bent / Bent No, Pier(s) #(-#) (e.g., "Bent 1, Piers 1-3")	Standard Penetration Test (SPT) EACH	Crosshole Sonic Logging (CSL) EACH	Sonic Caliper Testing (SCT) EACH	Shaft Inspection Device (SID) EACH	Pile Integrity Test (PIT) EACH
Bent 1, Piers 1-3		1	3	1	
TOTAL QUANTITY:		1	3	1	

NOTES:

- The Pile Foundation Tables are based on the bridge substructure design and foundation recommendations sealed by a North Carolina Professional Engineer (J.Jenkins, #050329) on 08-06-2025.
- Total Pile Driving Equipment Setup quantity (not shown in Pile Foundation Tables) equals the number of driven piles, i.e., the number of piles with a Required Driving Resistance.
- The Engineer may adjust the quantity for DPT Testing, Pipe Pile Plates, Permanent Steel Casing, SPTs, TIPs, SCTs, CSL Testing, SID Inspections and PITs when necessary.

PROJECT NO. DF18314.2075090

POLK COUNTY

STATION: 12+15.00 -L-

SHEET 3 OF 4

DRAWN BY : M. ACOSTA DATE : 08/2025
 CHECKED BY : T. R. LAWS DATE : 08/2025
 DESIGN ENGINEER OF RECORD: T. R. LAWS DATE : 08/2025

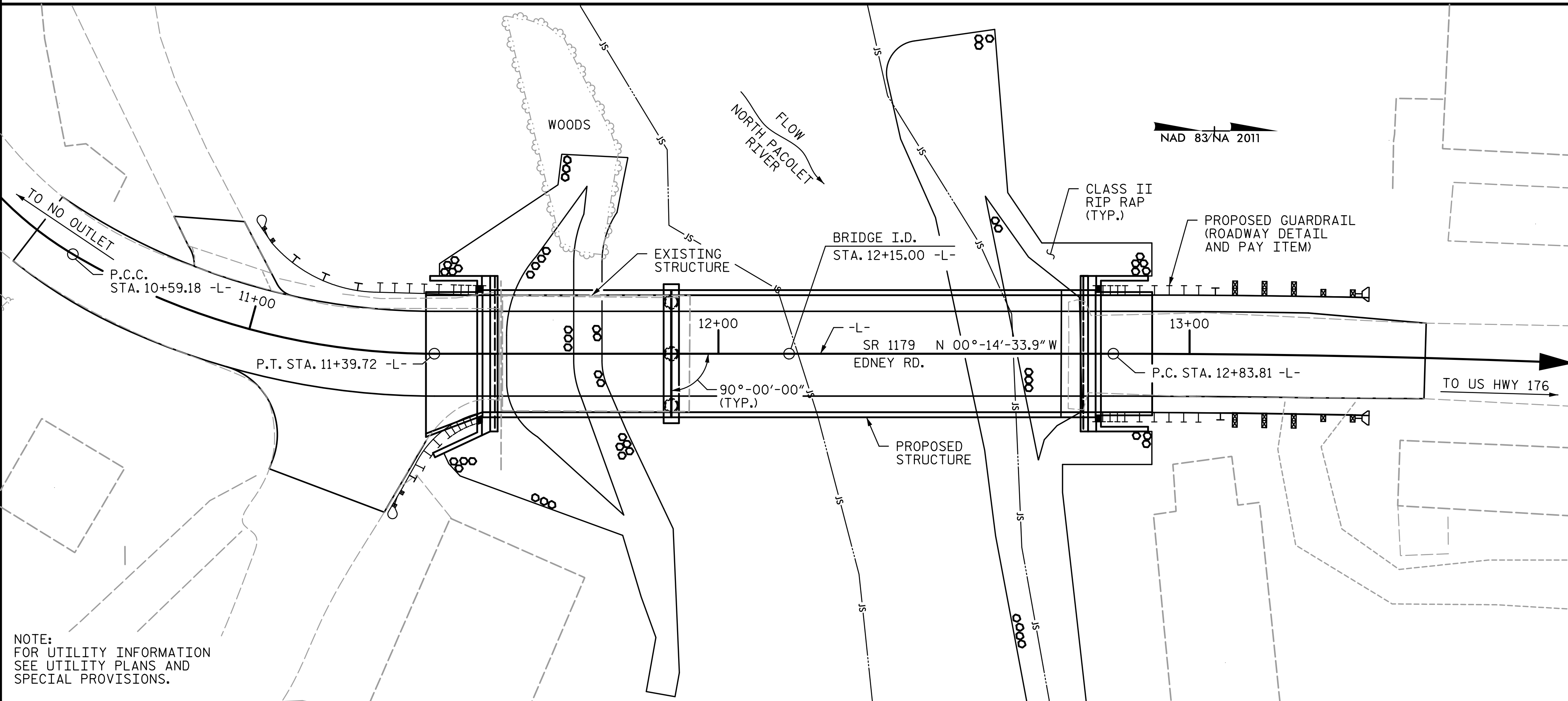
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10/7/2025
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 www.rsandh.com
 North Carolina License No. 50737-76463-C-28

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
**PILE AND DRILLED PIER
 FOUNDATION TABLES**

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-3
1			3			TOTAL SHEETS
2			4			24

BENCHMARK BM-1: RR SPIKE IN BASE OF 48" OAK. STA. 13+31.44 -L-, 76.00' RT. EL. 981.97 (N 552683, E 1023469)



NOTE:
FOR UTILITY INFORMATION
SEE UTILITY PLANS AND
SPECIAL PROVISIONS.

LOCATION SKETCH

NOTES

- ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.
- THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
- THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
- FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE "STANDARD NOTES" SHEET.
- FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.
- THE EXISTING STRUCTURE CONSISTING OF (3) 45'-0" SPAN WITH A CLEAR ROADWAY OF 30'-10" AND SUPPORTED BY REINFORCED CONCRETE ABUTMENTS SHALL BE REMOVED.
- REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.
- THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.
- THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES".
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.
- FOR ASBESTOS ASSESSMENT, SEE SPECIAL PROVISIONS.
- THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED A DISTANCE OF 20'-0" +/- TO THE LEFT AND TO THE RIGHT OF THE CENTERLINE OF THE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.
- FOR SCT TESTING, SEE GEOTECHNICAL SPECIAL PROVISIONS.

HYDRAULIC DATA

DESIGN DISCHARGE	= 2610 CFS
FREQUENCY OF DESIGN FLOOD	= 25 YRS
DESIGN HIGH WATER ELEVATION	= 974.60'
DRAINAGE AREA	= 17.2 SQ. MI.
BASE DISCHARGE (Q100)	= 3860 CFS
BASE HIGH WATER ELEVATION	= 975.90'

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE	= 12,000 CFS
FREQUENCY OF OVERTOPPING FLOOD	= 500+ YRS
* OVERTOPPING FLOOD ELEVATION	= 982.20'
* OVERTOPPING @ STA. 12+98.61 -L-	
WSEL TAKEN AT RIVER STATION 589+87	

PROJECT NO. DF18314.2075090

POLK COUNTY

STATION: 12+15.00 -L-

SHEET 4 OF 4

TOTAL BILL OF MATERIALS

	REMOVAL OF EXISTING STRUCTURE @ STA. 12+15.00 -L-	ASBESTOS ASSESSMENT	PILE EXCAVATION IN SOIL	3'-6" DIA. DRILLED PIERS	PERMANENT STEEL CASING FOR 3'-6" Ø DRILLED PIER	SID INSPECTIONS	CSL TESTING	UNCLASSIFIED STRUCTURE EXCAVATION @ STA. 12+15.00 -L-	CLASS A CONCRETE	BRIDGE APPROACH SLABS AT STA. 12+15.00 -L-	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12X53 STEEL PILES
	LUMP SUM	LUMP SUM	LIN. FT.	LIN. FT.	LIN. FT.	EACH	EACH	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	LBS.	EACH
SUPERSTRUCTURE													
END BENT NO. 1			50						22.8		3,345		5
BENT NO. 1				159	39	1	1		16.9		13,736	3,705	
END BENT NO. 2			50						22.3		3,143		5
TOTAL	LUMP SUM	LUMP SUM	100	159	39	1	1	LUMP SUM	62.0	LUMP SUM	20,224	3,705	10

	HP 12X53 STEEL PILES	STEEL PILE POINTS	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3'-0" X 2'-9" PRESTRESSED CONCRETE BOX BEAM	SCT TESTING		
	NO.	LIN. FT.	EACH	TONS	SQ. YDS.	LUMP SUM	NO.	LIN. FT.	EACH	
SUPERSTRUCTURE				260			18	1,170		
END BENT NO. 1	5	225	5	213	225					
BENT NO. 1									3	
END BENT NO. 2	5	225	5	417	440					
TOTAL	10	450	10	260	630	665	LUMP SUM	18	1,170	3

DRAWN BY : M. R. ACOSTA DATE : 02/2025
 CHECKED BY : T. R. LAWS DATE : 02/2025
 DESIGN ENGINEER OF RECORD: T. R. LAWS DATE : 10/2025

DOCUMENT NOT CONSIDERED
FINAL UNLESS ALL
SIGNATURES COMPLETED

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING
FOR BRIDGE ON SR 1179
(EDNEY RD.) OVER
NORTH PACOLET RIVER
BETWEEN SR 1180
AND US HWY 176

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-4
1			3			TOTAL SHEETS
2			4			24

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

LOAD TYPE	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE										SERVICE III LIMIT STATE					COMMENT NUMBER			
						MOMENT					SHEAR					MOMENT								
						LIVE-LOAD FACTORS (γ _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FT)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FT)	LIVE-LOAD FACTORS (γ _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN		GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FT)	
DESIGN LOAD	HL-93 (INVENTORY)	N/A	①	2.52	---	1.75	0.28	3.19	A	EL	19.25	0.54	2.52	A	ER	7.25	0.80	0.28	4.33	A	EL	19.25		
	HL-93 (OPERATING)	N/A		3.31	---	1.35	0.28	4.14	A	EL	19.25	0.54	3.31	A	ER	7.25	NA	--	--	--	--	--		
	HS-20 (INVENTORY)	36.000	②	2.91	104.76	1.75	0.28	3.94	A	EL	15.25	0.54	2.91	A	ER	7.25	0.80	0.28	5.45	A	EL	15.25		
	HS-20 (OPERATING)	36.000		3.82	137.52	1.35	0.28	5.10	A	EL	15.25	0.54	3.82	A	ER	7.25	NA	--	--	--	--	--		
LEGAL LOAD	SINGLE VEHICLE (SV)	SNSH	13.500		7.64	103.14	1.40	0.28	9.24	A	EL	19.25	0.54	7.64	A	ER	7.25	0.80	0.28	10.01	A	EL	19.25	
		SNGARBS2	20.000		5.77	115.40	1.40	0.28	7.44	A	EL	15.25	0.54	5.77	A	ER	7.25	0.80	0.28	8.23	A	EL	15.25	
		SNAGRIS2	22.000		5.51	121.22	1.40	0.28	7.28	A	EL	15.25	0.54	5.51	A	ER	7.25	0.80	0.28	8.05	A	EL	15.25	
		SNCOTTS3	27.250		3.77	102.73	1.40	0.28	4.62	A	EL	19.25	0.54	3.77	A	ER	7.25	0.80	0.28	5.01	A	EL	19.25	
		SNAGGRS4	34.925		3.38	118.05	1.40	0.28	4.17	A	EL	19.25	0.54	3.38	A	ER	7.25	0.80	0.28	4.52	A	EL	19.25	
		SNS5A	35.550		3.59	127.62	1.40	0.28	4.05	A	EL	19.25	0.54	3.59	A	ER	7.25	0.80	0.28	4.39	A	EL	19.25	
		SNS6A	39.950		3.36	134.23	1.40	0.28	3.86	A	EL	19.25	0.54	3.36	A	ER	7.25	0.80	0.28	4.18	A	EL	19.25	
	SNS7B	42.000		3.46	145.32	1.40	0.28	3.68	A	EL	19.25	0.54	3.46	A	ER	7.25	0.80	0.28	3.99	A	EL	19.25		
	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGRIT3	33.000		3.97	131.01	1.40	0.28	4.75	A	EL	19.25	0.54	3.97	A	ER	7.25	0.80	0.28	5.15	A	EL	19.25	
		TNT4A	33.075		3.72	123.04	1.40	0.28	4.79	A	EL	15.25	0.54	3.72	A	ER	7.25	0.80	0.28	5.22	A	EL	19.25	
		TNT6A	41.600		3.64	151.42	1.40	0.28	4.09	A	EL	19.25	0.54	3.64	A	ER	7.25	0.80	0.28	4.43	A	EL	19.25	
		TNT7A	42.000		3.38	141.96	1.40	0.28	4.19	A	EL	15.25	0.54	3.38	A	ER	7.25	0.80	0.28	4.55	A	EL	19.25	
		TNT7B	42.000		3.28	137.76	1.40	0.28	4.23	A	EL	15.25	0.54	3.28	A	ER	7.25	0.80	0.28	4.63	A	EL	19.25	
		TNAGRIT4	43.000		3.15	135.45	1.40	0.28	4.07	A	EL	15.25	0.54	3.15	A	ER	7.25	0.80	0.28	4.50	A	EL	15.25	
TNAGRT5A		45.000		3.36	151.20	1.40	0.28	3.86	A	EL	19.25	0.54	3.36	A	ER	7.25	0.80	0.28	4.18	A	EL	19.25		
TNAGRT5B	45.000		③	2.96	133.20	1.40	0.28	3.75	A	EL	19.25	0.54	2.96	A	ER	7.25	0.80	0.28	4.06	A	EL	19.25		
EMERGENCY VEHICLE (EV)	EV2	28.750		4.37	125.64	1.30	0.28	5.73	A	EL	15.25	0.54	4.37	A	ER	7.25	0.80	0.28	5.89	A	EL	15.25		
	EV3	43.000		④	2.93	125.99	1.30	0.28	3.82	A	EL	19.25	0.54	2.93	A	ER	7.25	0.80	0.28	3.84	A	EL	19.25	

LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	γ _{DC}	γ _{DW}
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

NOTES:

COMMENTS:

-
-
-
-

④ CONTROLLING LOAD RATING

① DESIGN LOAD RATING (HL-93)

② DESIGN LOAD RATING (HS-20)

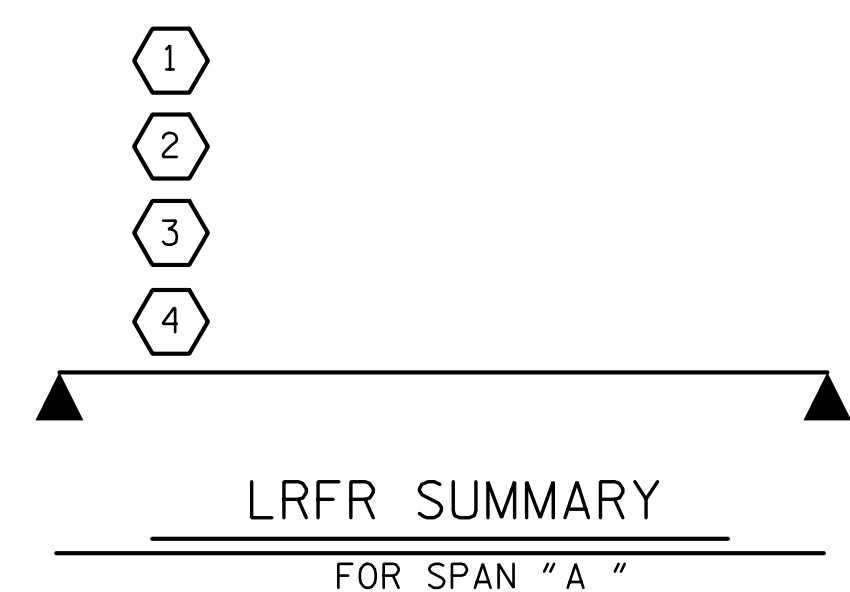
③ LEGAL LOAD RATING **

④ EMERGENCY VEHICLE LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER
EL - EXTERIOR LEFT GIRDER
ER - EXTERIOR RIGHT GIRDER



PROJECT NO. DF18314.2075090
POLK COUNTY
 STATION: 12+15.00 -L-

SHEET 1 OF 2



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 LRFR SUMMARY FOR
 40' BOX BEAM UNIT
 90° SKEW
 (NON-INTERSTATE TRAFFIC)

DRAWN BY : J. SCACCA DATE : 02/2025
 CHECKED BY : M. KHIN DATE : 02/2025
 DESIGN ENGINEER OF RECORD: T. R. LAWS DATE : 10/2025

DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-5
1			3			TOTAL SHEETS
2			4			24

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

LOAD TYPE	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING #	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE										SERVICE III LIMIT STATE					COMMENT NUMBER			
						MOMENT					SHEAR					MOMENT								
						LIVE-LOAD FACTORS (γ _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FT)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FT)	LIVE-LOAD FACTORS (γ _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN		GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FT)	
DESIGN LOAD	HL-93 (INVENTORY)	N/A	1	1.109	--	1.75	0.272	1.47	B	EL	44.250	0.493	1.26	B	EL	4.425	0.80	0.272	1.11	B	EL	44.250		
	HL-93 (OPERATING)	N/A		1.633	--	1.35	0.272	1.90	B	EL	44.250	0.493	1.63	B	EL	4.425	N/A	--	--	--	--	--		
	HS-20 (INVENTORY)	36.000	2	1.507	54.255	1.75	0.272	1.99	B	EL	44.250	0.493	1.65	B	EL	4.425	0.80	0.272	1.51	B	EL	44.250		
	HS-20 (OPERATING)	36.000		2.140	77.039	1.35	0.272	2.59	B	EL	44.250	0.493	2.14	B	EL	4.425	N/A	--	--	--	--	--		
LEGAL LOAD	SINGLE VEHICLE (SV)	SNSH	13.500		3.519	47.501	1.40	0.272	5.82	B	EL	44.250	0.493	5.05	B	EL	4.425	0.80	0.272	3.52	B	EL	44.250	
		SNGARBS2	20.000		2.572	51.43	1.40	0.272	4.25	B	EL	44.250	0.493	3.55	B	EL	4.425	0.80	0.272	2.57	B	EL	44.250	
		SNAGRIS2	22.000		2.415	53.122	1.40	0.272	4.00	B	EL	44.250	0.493	3.27	B	EL	4.425	0.80	0.272	2.41	B	EL	44.250	
		SNCOTTS3	27.250		1.749	47.674	1.40	0.272	2.89	B	EL	44.250	0.493	2.52	B	EL	4.425	0.80	0.272	1.75	B	EL	44.250	
		SNAGGRS4	34.925		1.443	50.381	1.40	0.272	2.39	B	EL	44.250	0.493	2.06	B	EL	4.425	0.80	0.272	1.44	B	EL	44.250	
		SNS5A	35.550		1.412	50.195	1.40	0.272	2.34	B	EL	44.250	0.493	2.07	B	EL	4.425	0.80	0.272	1.41	B	EL	44.250	
		SNS6A	39.950		1.287	51.435	1.40	0.272	2.13	B	EL	44.250	0.493	1.88	B	EL	4.425	0.80	0.272	1.29	B	EL	44.250	
	SNS7B	42.000		1.226	51.483	1.40	0.272	2.03	B	EL	44.250	0.493	1.83	B	EL	4.425	0.80	0.272	1.23	B	EL	44.250		
	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGRIT3	33.000		1.568	51.733	1.40	0.272	2.59	B	EL	44.250	0.493	2.24	B	EL	4.425	0.80	0.272	1.57	B	EL	44.250	
		TNT4A	33.075		1.572	52.007	1.40	0.272	2.60	B	EL	44.250	0.493	2.20	B	EL	4.425	0.80	0.272	1.57	B	EL	44.250	
		TNT6A	41.600		1.278	53.170	1.40	0.272	2.11	B	EL	44.250	0.493	1.92	B	EL	4.425	0.80	0.272	1.28	B	EL	44.250	
		TNT7A	42.000		1.281	53.782	1.40	0.272	2.12	B	EL	44.250	0.493	1.89	B	EL	4.425	0.80	0.272	1.28	B	EL	44.250	
		TNT7B	42.000		1.315	55.229	1.40	0.272	2.18	B	EL	44.250	0.493	1.79	B	EL	4.425	0.80	0.272	1.31	B	EL	44.250	
		TNAGRIT4	43.000		1.258	54.101	1.40	0.272	2.08	B	EL	44.250	0.493	1.74	B	EL	4.425	0.80	0.272	1.26	B	EL	44.250	
TNAGRT5A		45.000		1.190	53.537	1.40	0.272	1.97	B	EL	44.250	0.493	1.71	B	EL	4.425	0.80	0.272	1.19	B	EL	44.250		
TNAGRT5B	45.000		3	1.178	53.027	1.40	0.272	1.95	B	EL	44.250	0.493	1.66	B	EL	4.425	0.80	0.272	1.18	B	EL	44.250		
EMERGENCY VEHICLE (EV)	EV2	28.750		2.296	66.005	1.30	0.272	3.25	B	EL	44.250	0.493	2.49	B	EL	4.425	0.80	0.272	2.30	B	EL	44.250		
	EV3	43.000		4	1.510	64.924	1.30	0.272	2.14	B	EL	44.250	0.493	1.67	B	EL	4.425	0.80	0.272	1.51	B	EL	44.250	

LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	γ _{DC}	γ _{DW}
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

NOTES:

COMMENTS:

-
-
-
-

CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

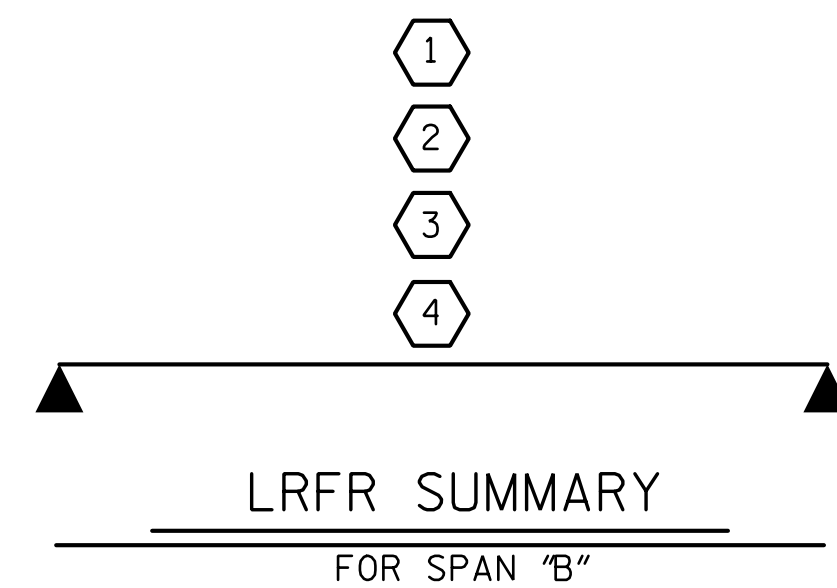
3 LEGAL LOAD RATING **

4 EMERGENCY VEHICLE LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER
EL - EXTERIOR LEFT GIRDER
ER - EXTERIOR RIGHT GIRDER



PROJECT NO. DF18314.2075090
POLK COUNTY
 STATION: 12+15.00 -L-

SHEET 2 OF 2

DRAWN BY : J. SCACCA DATE : 02/2025
 CHECKED BY : M. KHIN DATE : 02/2025
 DESIGN ENGINEER OF RECORD: T. R. LAWS DATE : 10/2025

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 www.rsandh.com
 North Carolina License No. 50973-75493-1-C28

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH						SHEET NO. S-6	
STANDARD LRFR SUMMARY FOR 90' BOX BEAM UNIT 90° SKEW (NON-INTERSTATE TRAFFIC)						TOTAL SHEETS 24	
REVISIONS							
NO.	BY:	DATE:	NO.	BY:	DATE:		
1			3				
2			4				

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BOX BEAMS.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 2 1/2" Ø DOWEL HOLES AT FIXED ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 4400 PSI IN SPAN A AND 6000 PSI IN SPAN B.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO BOX BEAM UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE LOCATION OF THE VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING STEEL.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

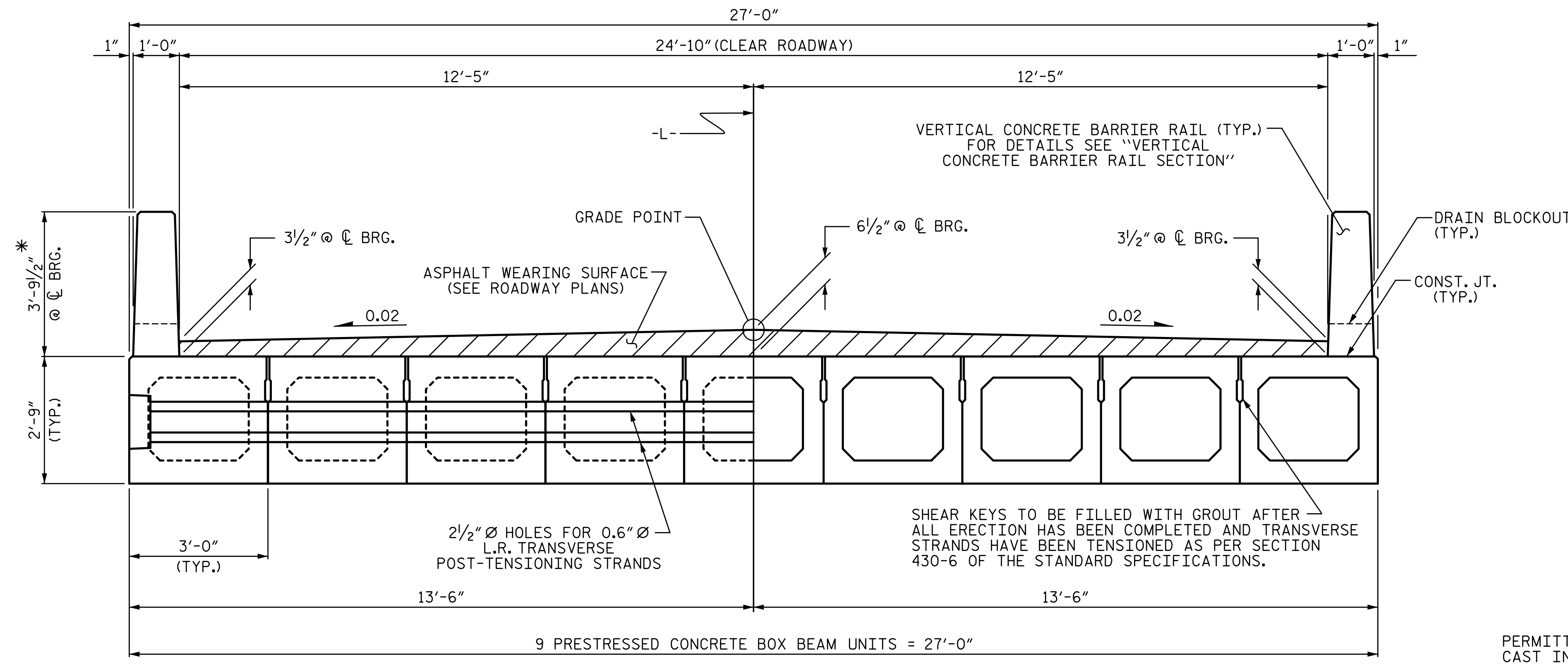
THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

THE DRAIN OPENING AT THE GUTTERLINE SHALL BE 4" X 5". THE HEIGHT OF THE BLOCKOUT IN THE VERTICAL CONCRETE BARRIER RAIL SHALL EXTEND FROM THE TOP OF THE BOX BEAM UNIT TO THE TOP OF THE DRAIN OPENING.

APPLY EPOXY PROTECTIVE COATING TO EXTERIOR FACE OF THE EXTERIOR BOX BEAM UNITS THAT REQUIRE DRAINS IN THE BARRIER RAIL.

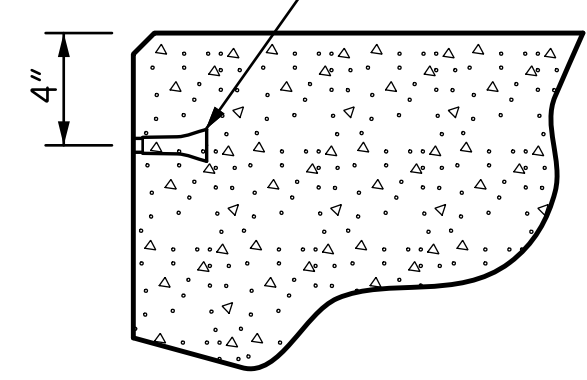


HALF SECTION AT INTERMEDIATE DIAPHRAGMS HALF SECTION THROUGH VOIDS

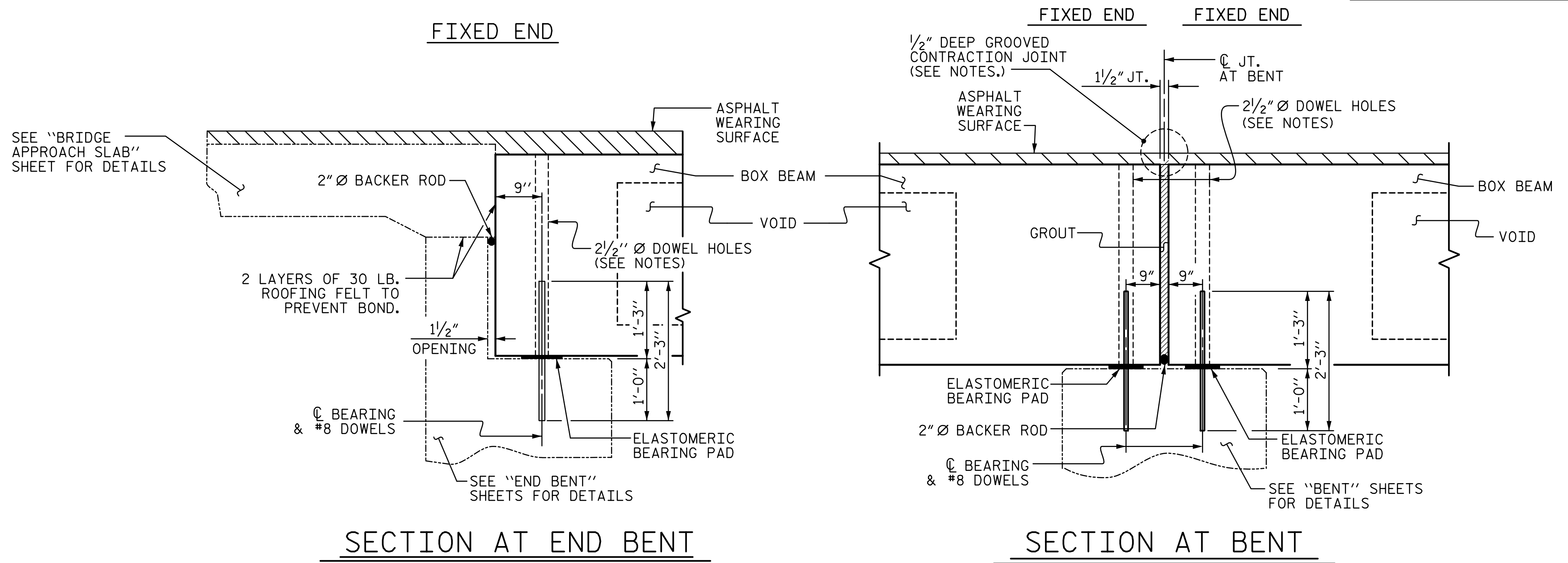
TYPICAL SECTION

*THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS, SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.

PERMITTED THREADED INSERT CAST IN OUTSIDE FACE OF EXTERIOR UNIT AND RECESSED 3/8" SIZE TO BE DETERMINED BY CONTRACTOR.



THREADED INSERT DETAIL

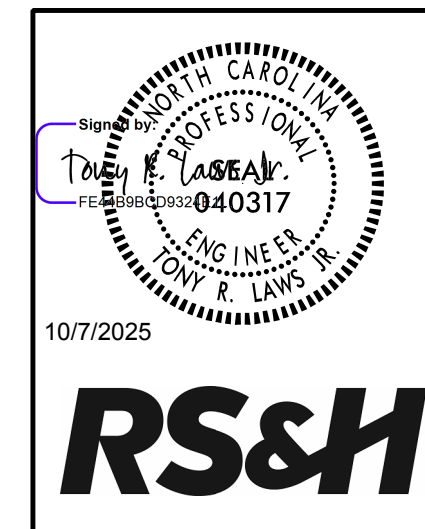


SECTION AT END BENT

SECTION AT BENT

PROJECT NO. DF18314.2075090
POLK COUNTY
 STATION: 12+15.00 -L-

SHEET 1 OF 7



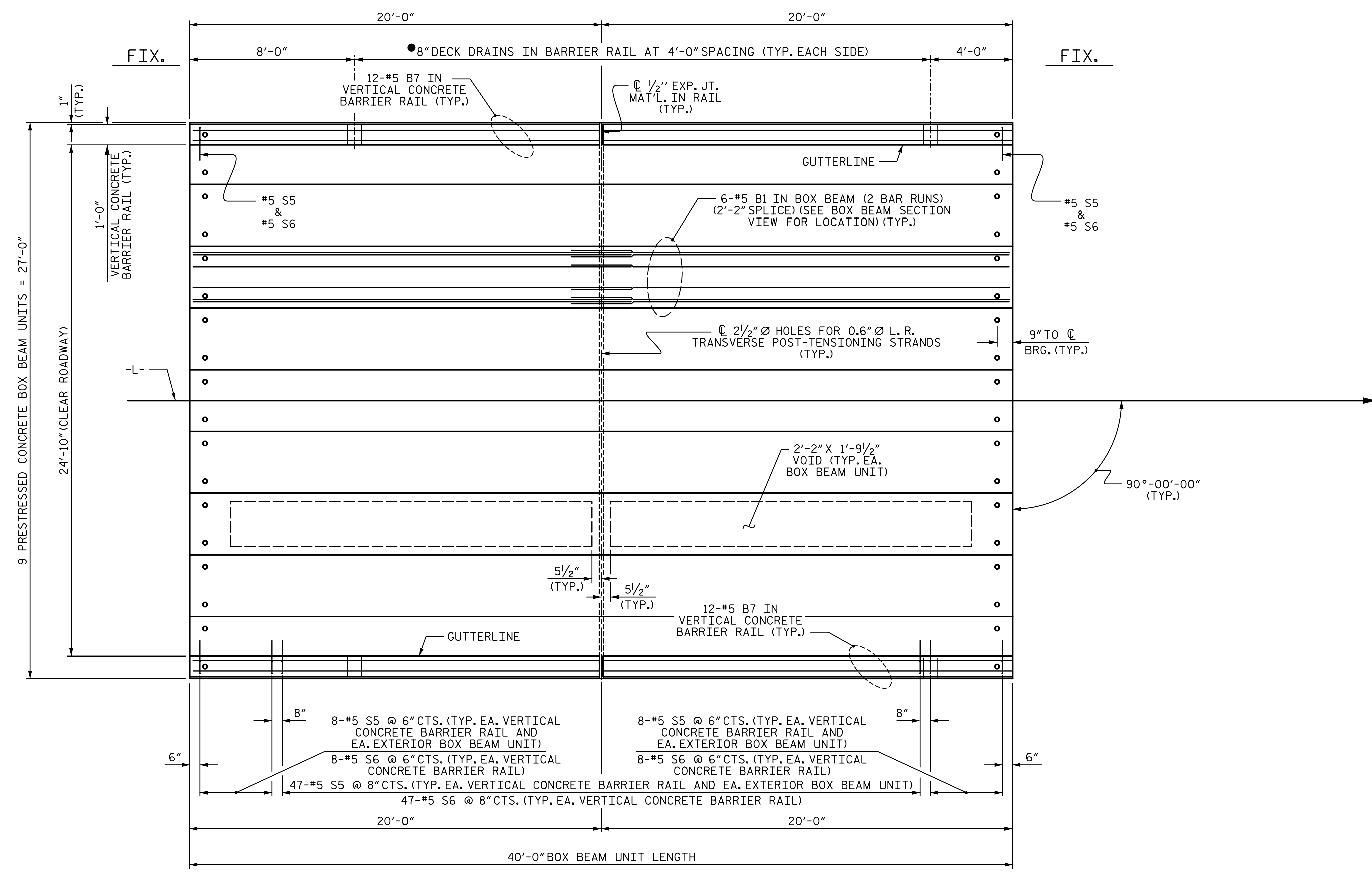
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 3'-0" X 2'-9"
 PRESTRESSED CONCRETE
 BOX BEAM UNIT
 90° SKEW

RS&H
 RS&H Architects-Engineers-Planners, Inc.
 8521 Six Forks Road, Suite 400
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 North Carolina License No. 50773-75493-C-28

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-7
1			3			TOTAL SHEETS 24
2			4			

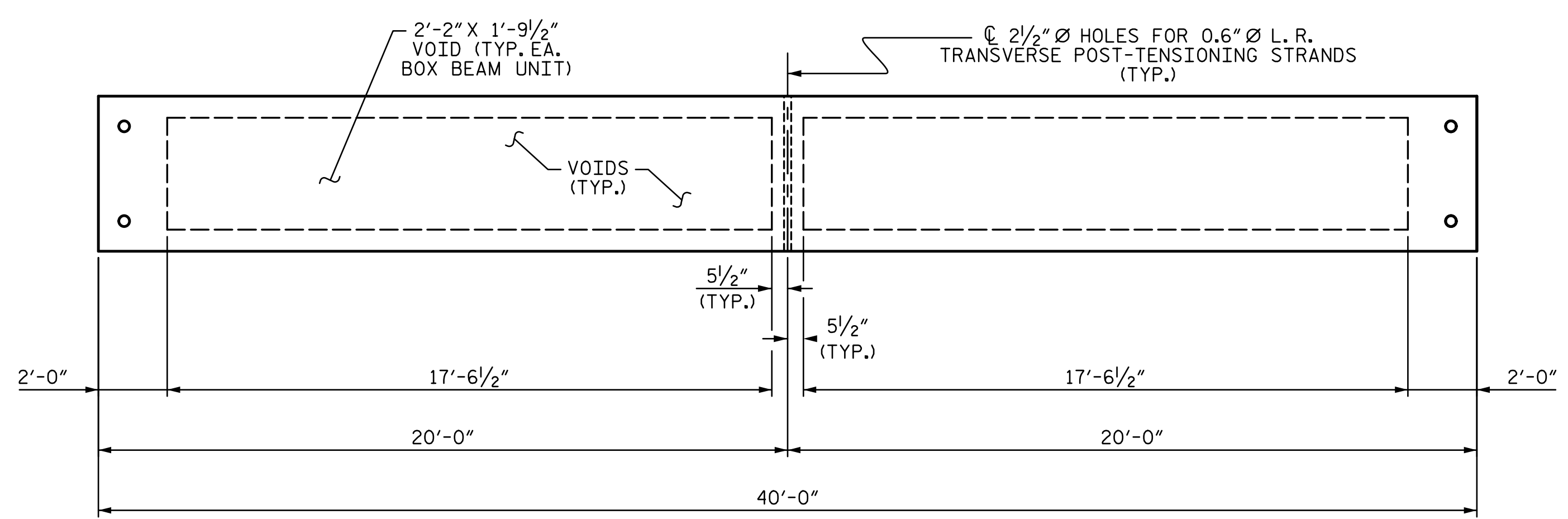
DRAWN BY : T. R. LAWS DATE : 02/2025
 CHECKED BY : V. SELLERS DATE : 02/2025
 DESIGN ENGINEER OF RECORD: T. R. LAWS DATE : 10/2025

DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED



● DRAIN BLOCKOUTS MAY BE SHIFTED AS NECESSARY TO AVOID BARRIER RAIL REINFORCING.

PLAN OF UNIT



DIAPHRAGM AND VOID LAYOUT

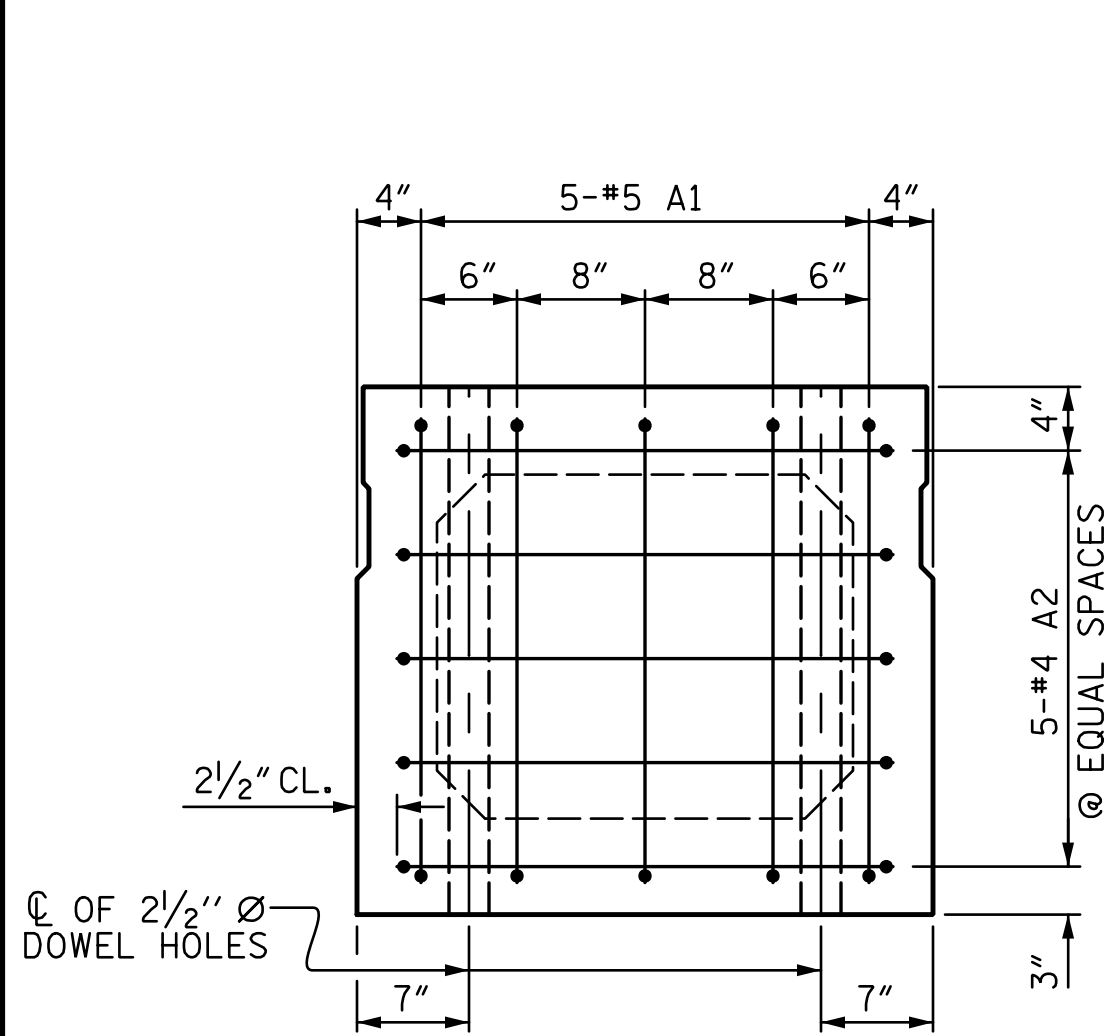
PROJECT NO. DF18314.2075090
POLK COUNTY
 STATION: 12+15.00 -L-

SHEET 2 OF 7

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
PLAN OF 40' UNIT 24'-10" CLEAR ROADWAY 90° SKEW SPAN A					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO.					S-8
TOTAL SHEETS					24

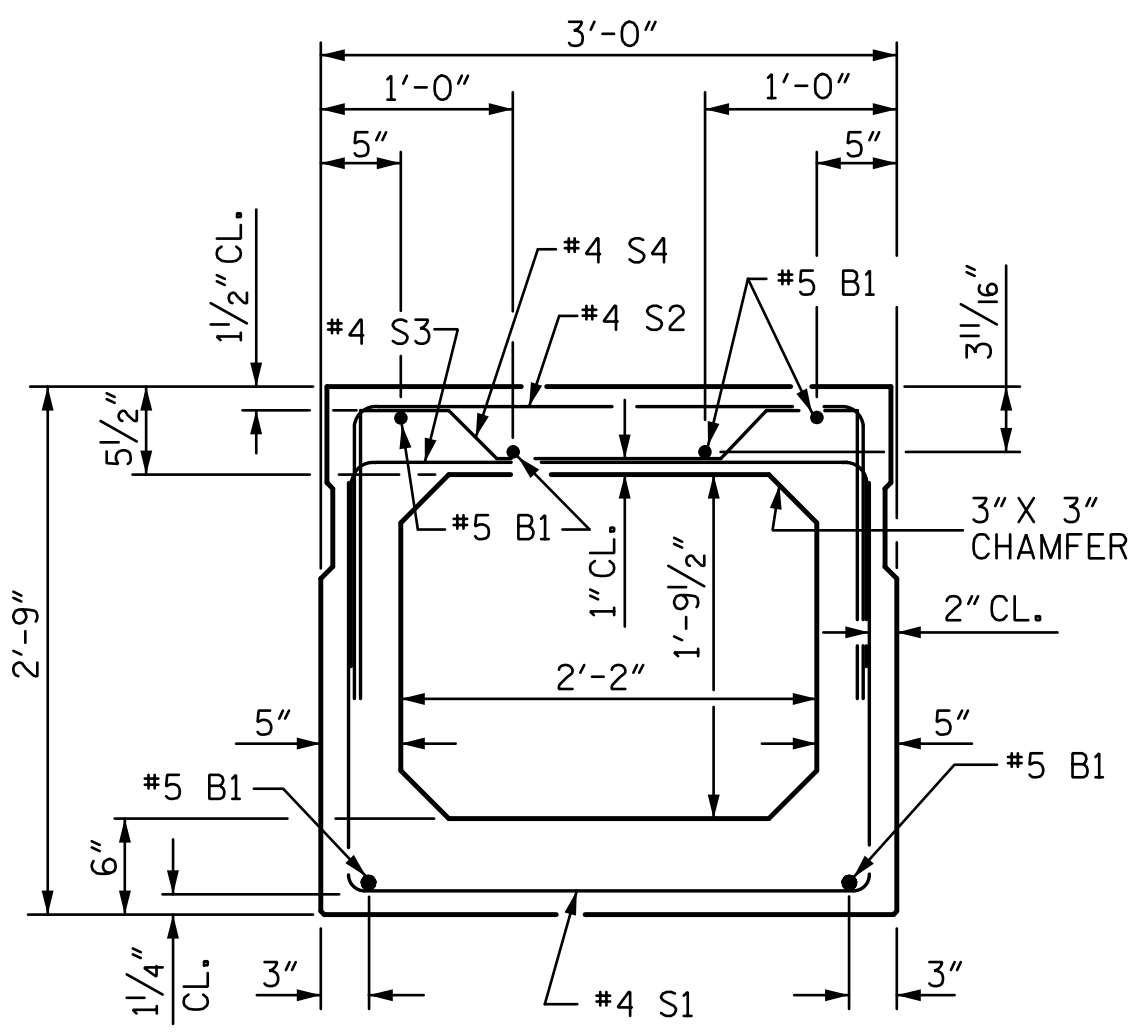
DRAWN BY : T. R. LAWS DATE : 02/2025
 CHECKED BY : V. SELLERS DATE : 02/2025
 DESIGN ENGINEER OF RECORD: T. R. LAWS DATE : 10/2025

DOCUMENT NOT CONSIDERED
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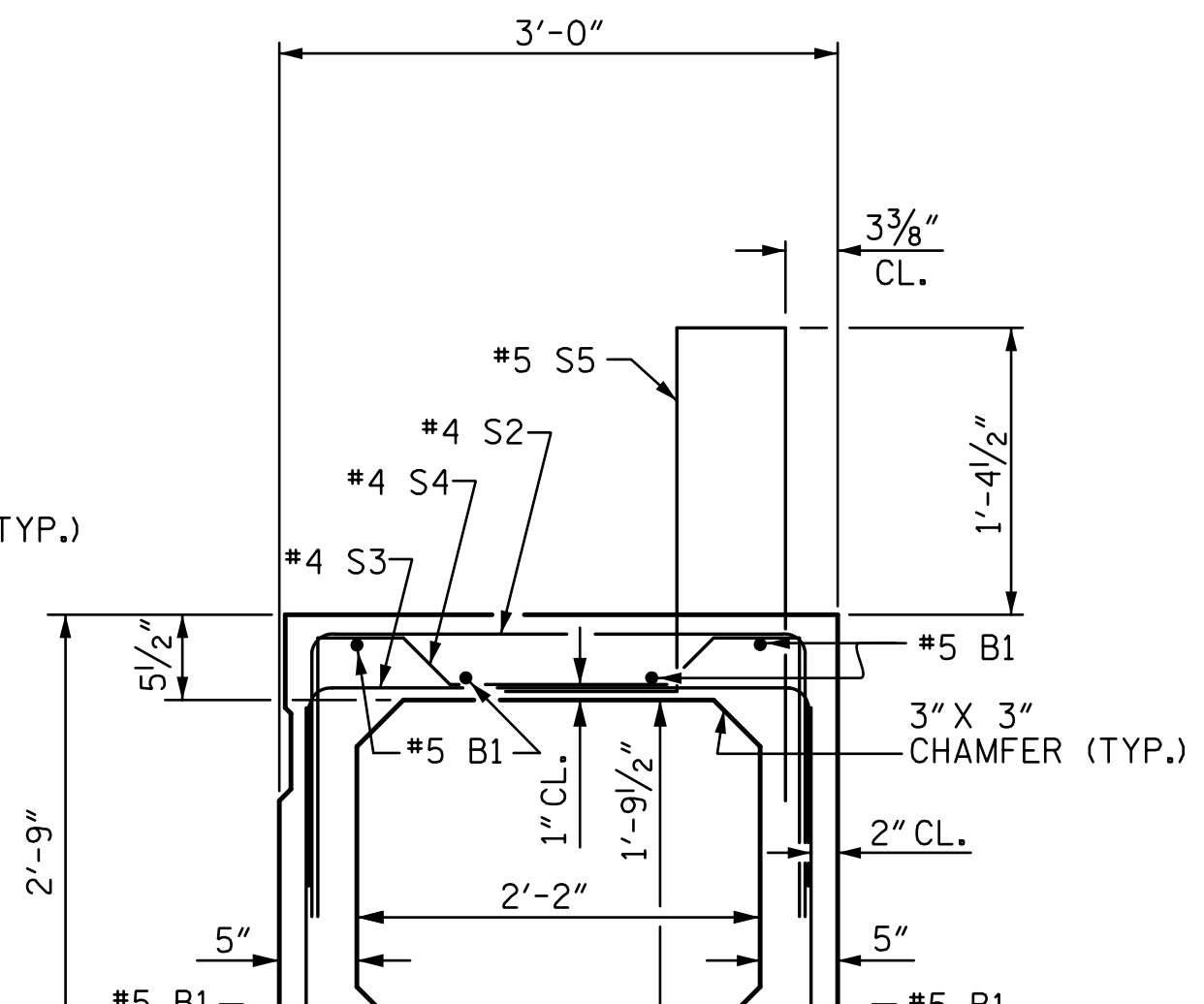
END ELEVATION

SHOWING PLACEMENT OF #5 & #4 "A" BARS AND LOCATION OF DOWEL HOLES. (INTERIOR BOX BEAM SECTION SHOWN-EXTERIOR SECTION SIMILAR EXCEPT SHEAR KEY LOCATION. STRAND LAYOUT NOT SHOWN.)



INTERIOR BOX BEAM SECTION

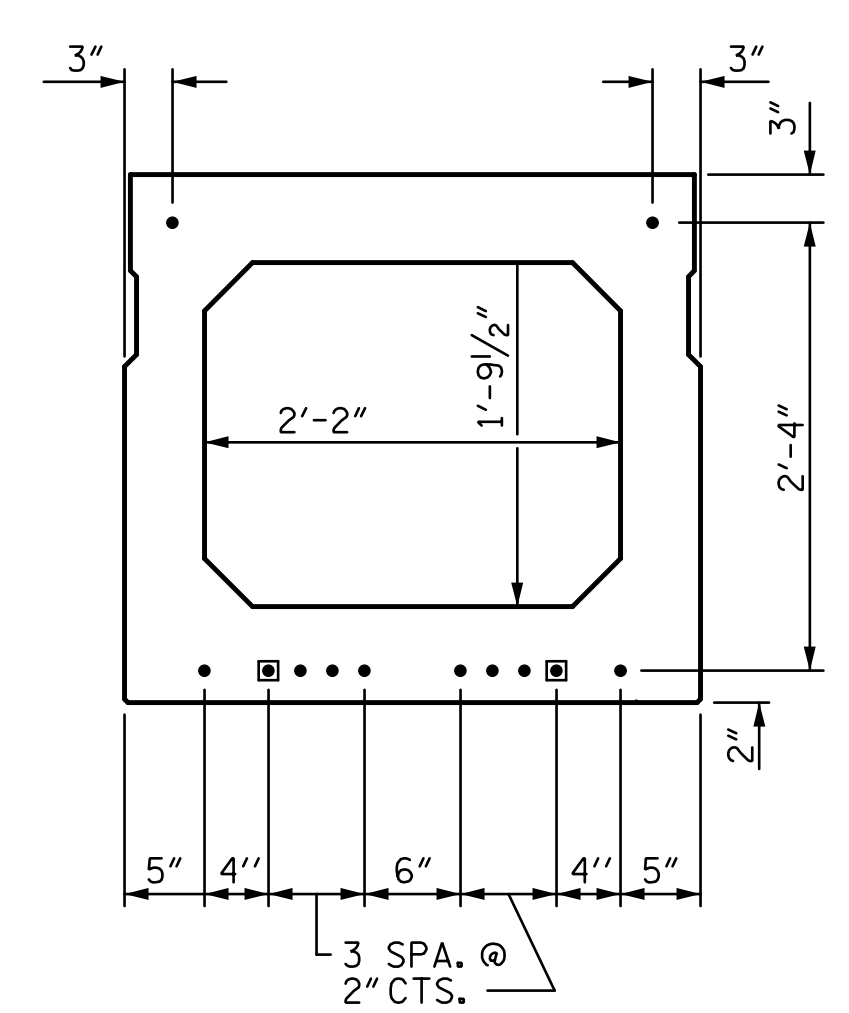
(STRAND LAYOUT NOT SHOWN)



EXTERIOR BOX BEAM SECTION

(STRAND LAYOUT NOT SHOWN)

0.6" Ø LOW RELAXATION STRAND LAYOUT



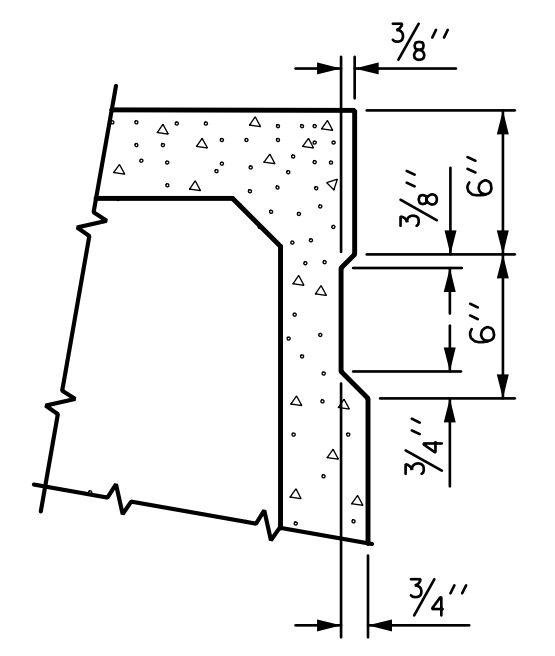
TYPICAL STRAND LOCATION

(12 STRANDS REQUIRED)

DEBONDING LEGEND

- FULLY BONDED STRANDS
- ◻ STRANDS DEBONDED FOR 4'-0" FROM END OF GIRDER

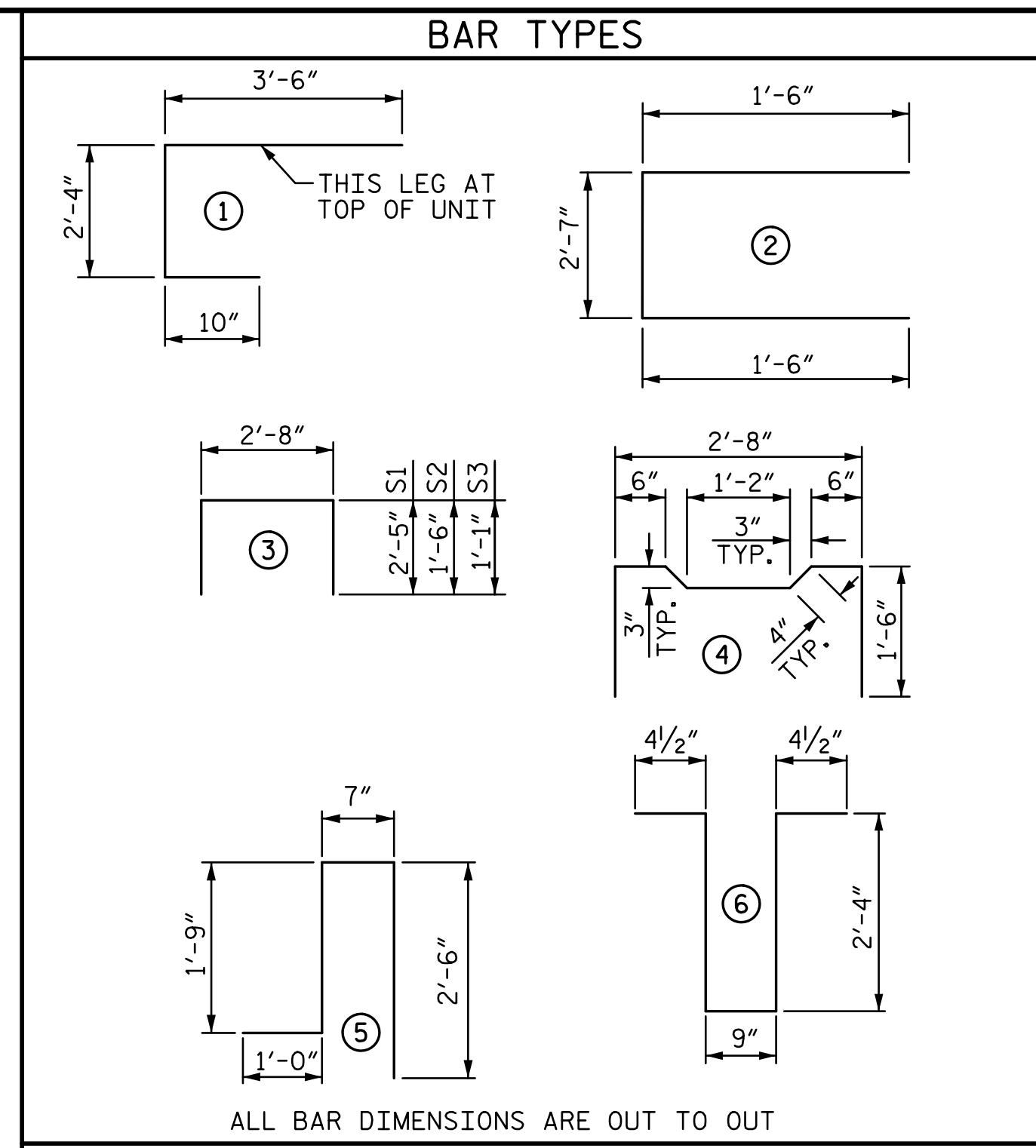
BOND SHALL BE BROKEN ON STRANDS AS SHOWN FOR THE SPECIFIED LENGTH FROM EACH END OF THE BOX BEAM. SEE STANDARD SPECIFICATIONS ARTICLE 1078-7.



SHEAR KEY DETAIL

NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR BOX BEAMS.

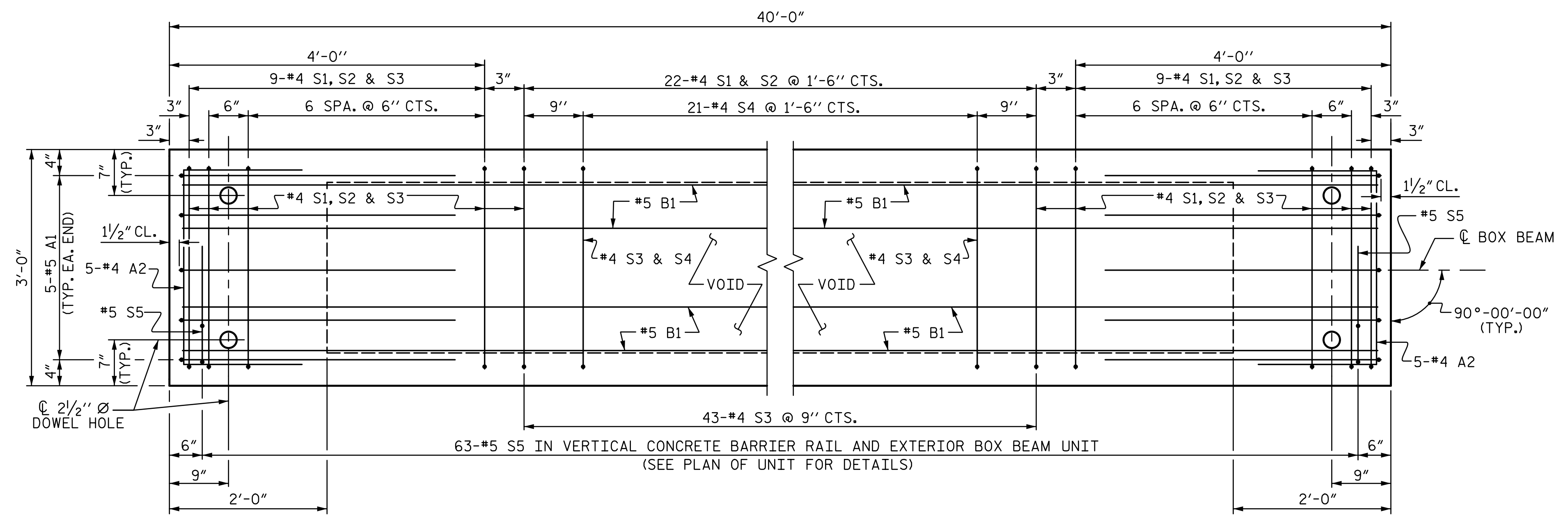
GRADE 270 STRANDS	
	0.6" Ø L.R.
AREA (SQUARE INCHES)	0.217
ULTIMATE STRENGTH (LBS. PER STRAND)	58,600
APPLIED PRESTRESS (LBS. PER STRAND)	43,950



ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL FOR ONE BOX BEAM SECTION

BAR	NUMBER	SIZE	TYPE	EXTERIOR UNIT		INTERIOR UNIT	
				LENGTH	WEIGHT	LENGTH	WEIGHT
A1	10	#5	1	6'-8"	70	6'-8"	70
A2	16	#4	2	5'-7"	60	5'-7"	60
B1	12	#5	STR	20'-6"	257	20'-6"	257
K1	3	#4	6	6'-2"	12	6'-2"	12
K2	2	#4	STR	2'-7"	3	2'-7"	3
S1	40	#4	3	7'-6"	200	7'-6"	200
S2	40	#4	3	5'-8"	151	5'-8"	151
S3	61	#4	3	4'-10"	197	4'-10"	197
S4	21	#4	4	5'-10"	82	5'-10"	82
* S5	63	#5	5	5'-10"	383	--	--
REINFORCING STEEL				1032	LBS.	1032	LBS.
* EPOXY COATED REINF. STEEL				383	LBS.		
5500 P.S.I. CONCRETE				7.3	CU. YDS.	7.2	CU. YDS.
0.6" Ø L.R. STRANDS				No. 12		No. 12	



PLAN OF BOX BEAM

EXTERIOR UNIT SHOWN, INTERIOR UNIT SIMILAR EXCEPT OMIT #5 S5 BARS. FOR LOCATION OF DIAPHRAGMS, SEE "PLAN OF UNIT". FOR THREADED INSERTS, SEE "THREADED INSERT DETAIL". FOR REINFORCING STEEL IN DIAPHRAGMS, SEE "DOUBLE DIAPHRAGM DETAILS".

PROJECT NO. DF18314.2075090
POLK COUNTY
 STATION: 12+15.00 -L-

SHEET 3 OF 7

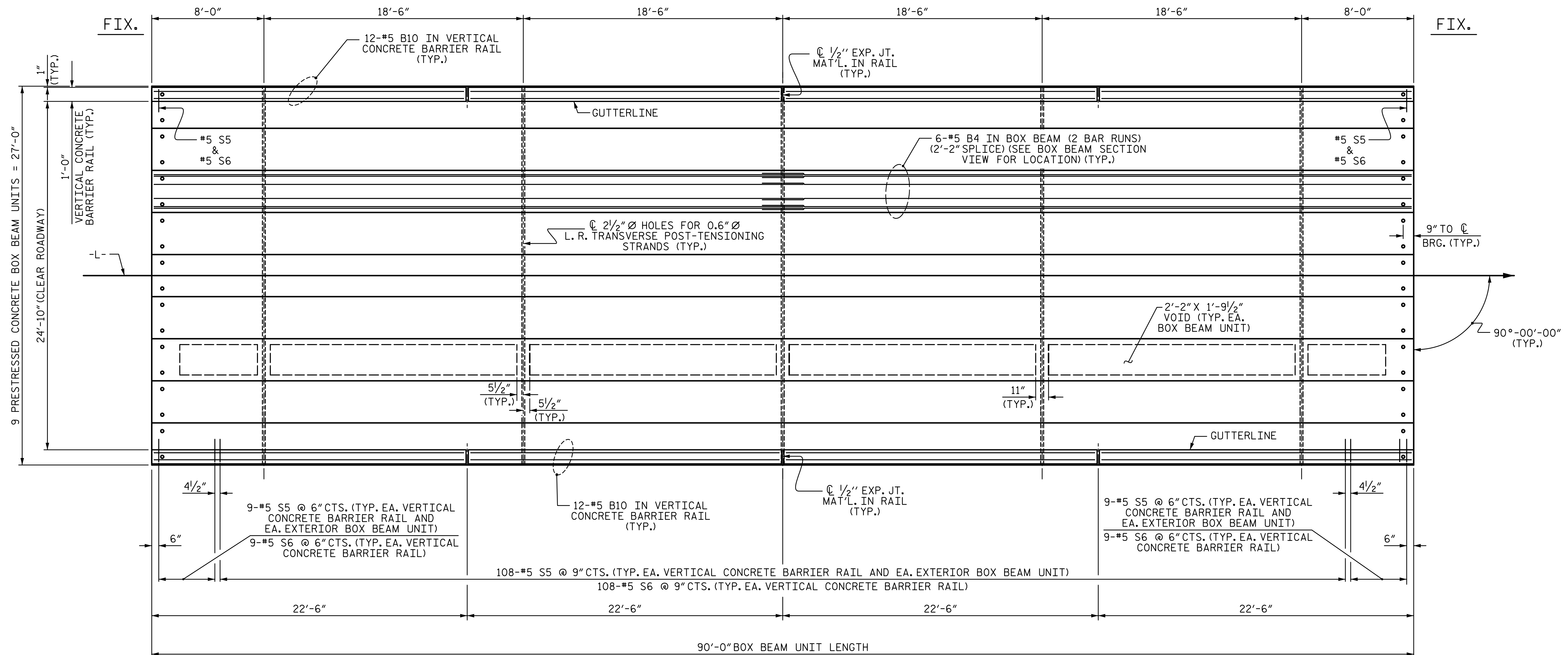
10/7/2025
RS&H
 RS&H Architects-Engineers-Planners, Inc.
 8521 Six Forks Road, Suite 400
 Raleigh, NC 27615
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 North Carolina License No. 50773-75493-1-C28

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
3'-0" X 2'-9" PRESTRESSED CONCRETE BOX BEAM UNIT SPAN A					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

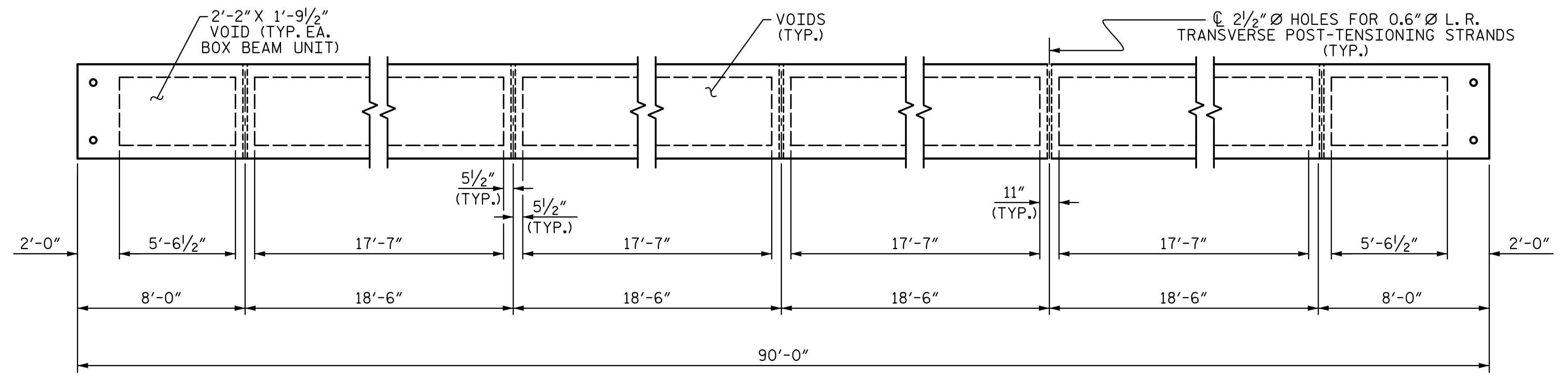
SHEET NO.	
S-9	TOTAL SHEETS 24

DRAWN BY :	T. R. LAWS	DATE :	02/2025
CHECKED BY :	V. SELLERS	DATE :	02/2025
DESIGN ENGINEER OF RECORD:	T. R. LAWS	DATE :	10/2025

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



PLAN OF UNIT



DIAPHRAGM AND VOID LAYOUT

PROJECT NO. DF18314.2075090
POLK COUNTY
 STATION: 12+15.00 -L-

SHEET 4 OF 7

10/7/2025

RS&H

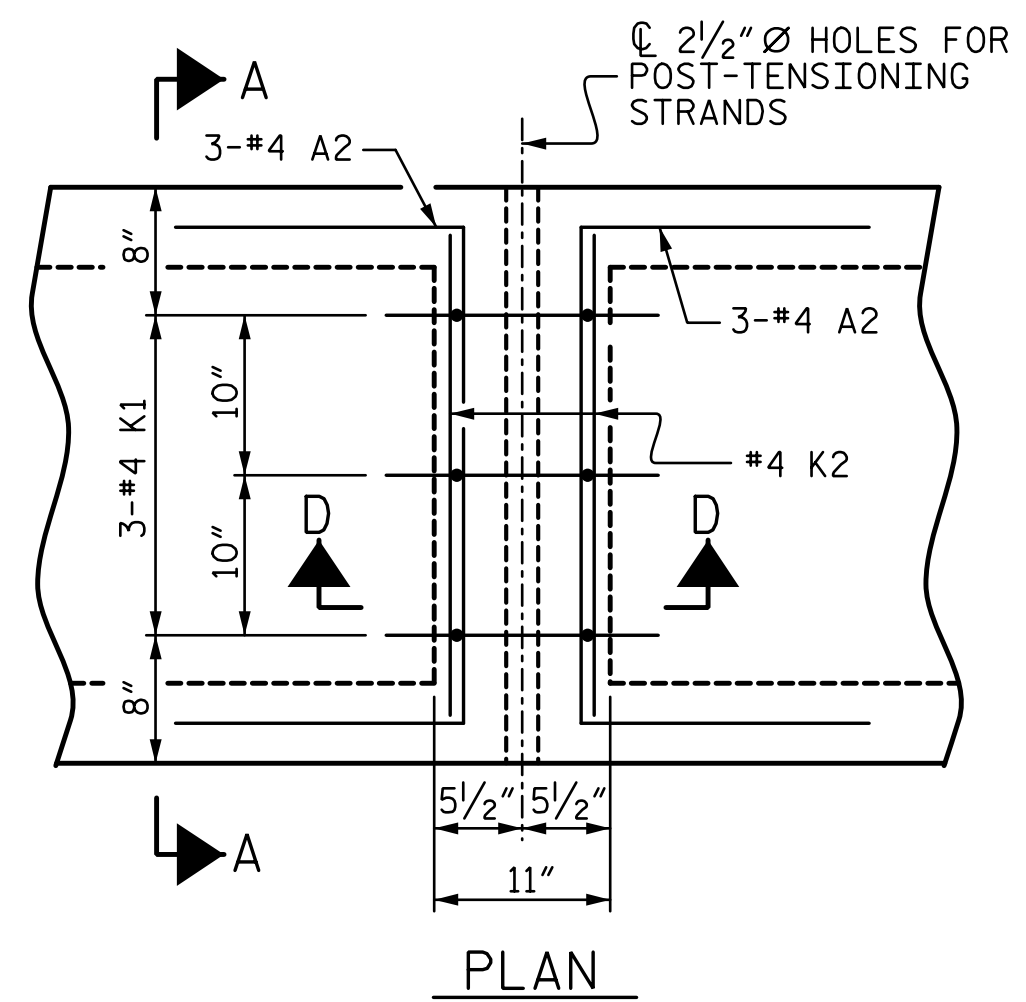
RS&H Architects-Engineers-Planners, Inc.
 8521 Six Forks Road, Suite 400
 Raleigh, NC 27615
 919-926-4100 FAX 919-846-9080
 www.rsandh.com
 North Carolina License No. 50737-26463-1-C28

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 PLAN OF 90' UNIT
 24'-10" CLEAR ROADWAY
 90° SKEW
 SPAN B

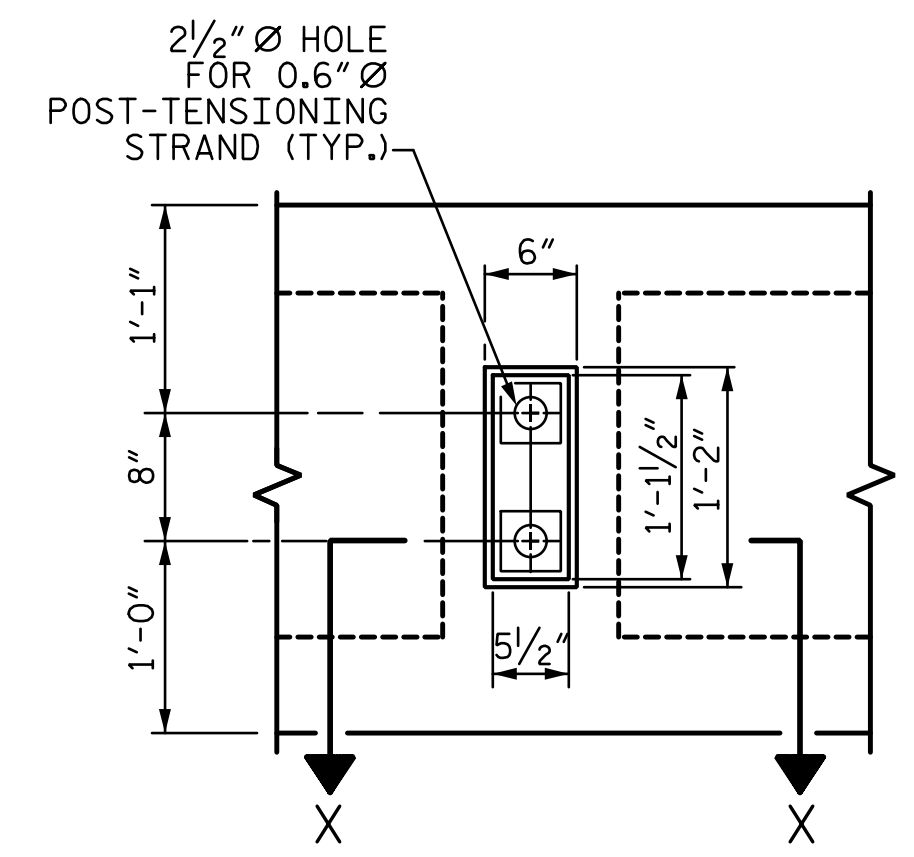
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CHECKED BY :	V. SELLERS	DATE :	02/2025
DRAWN BY :	DGE 8/II	REV. 8/14	MAA/TMG
CHECKED BY :	TMG II/II		

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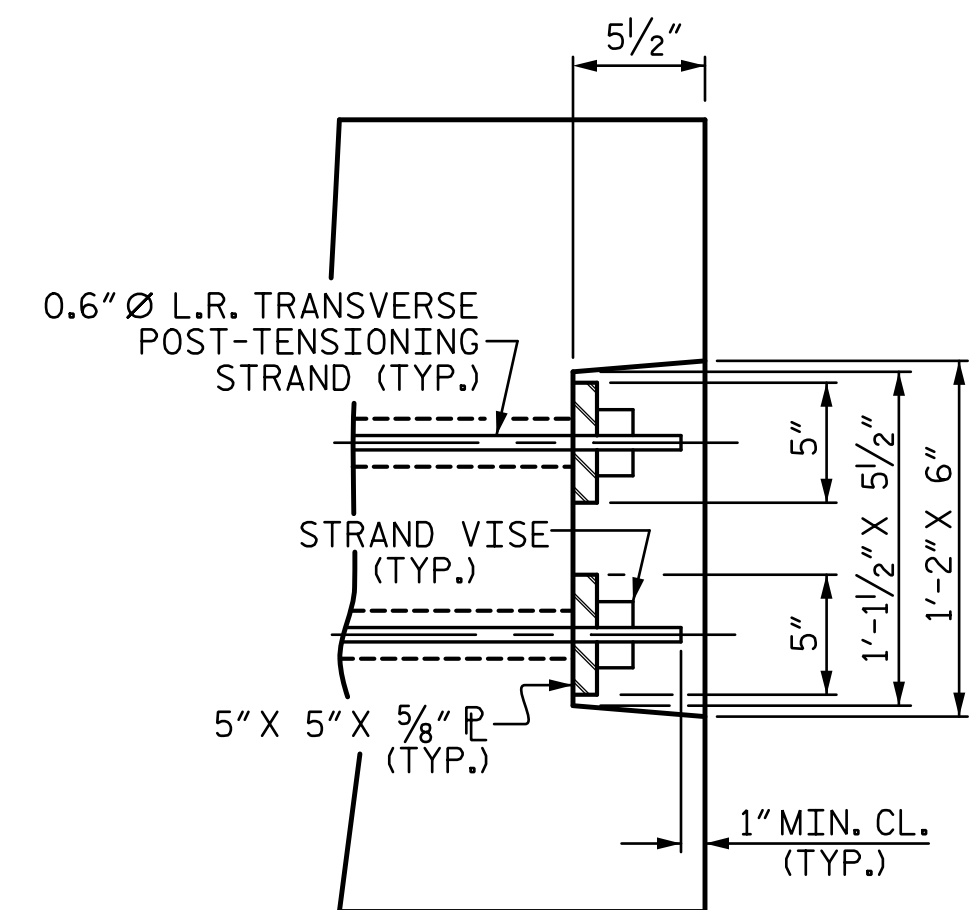
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NO.	BY:	DATE:	NO.	BY:	DATE:	S-10
1			3			TOTAL SHEETS
2			4			24



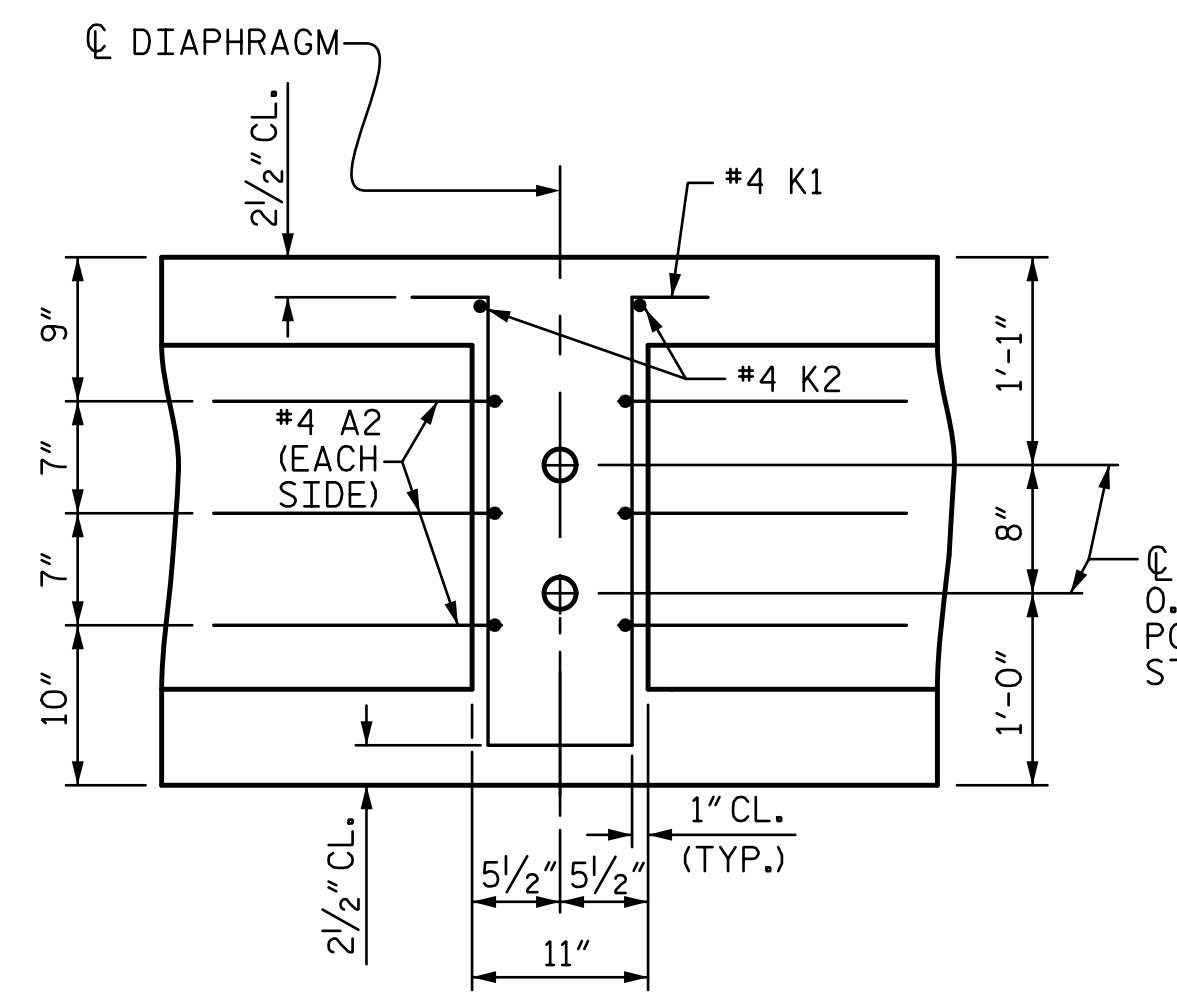
PLAN



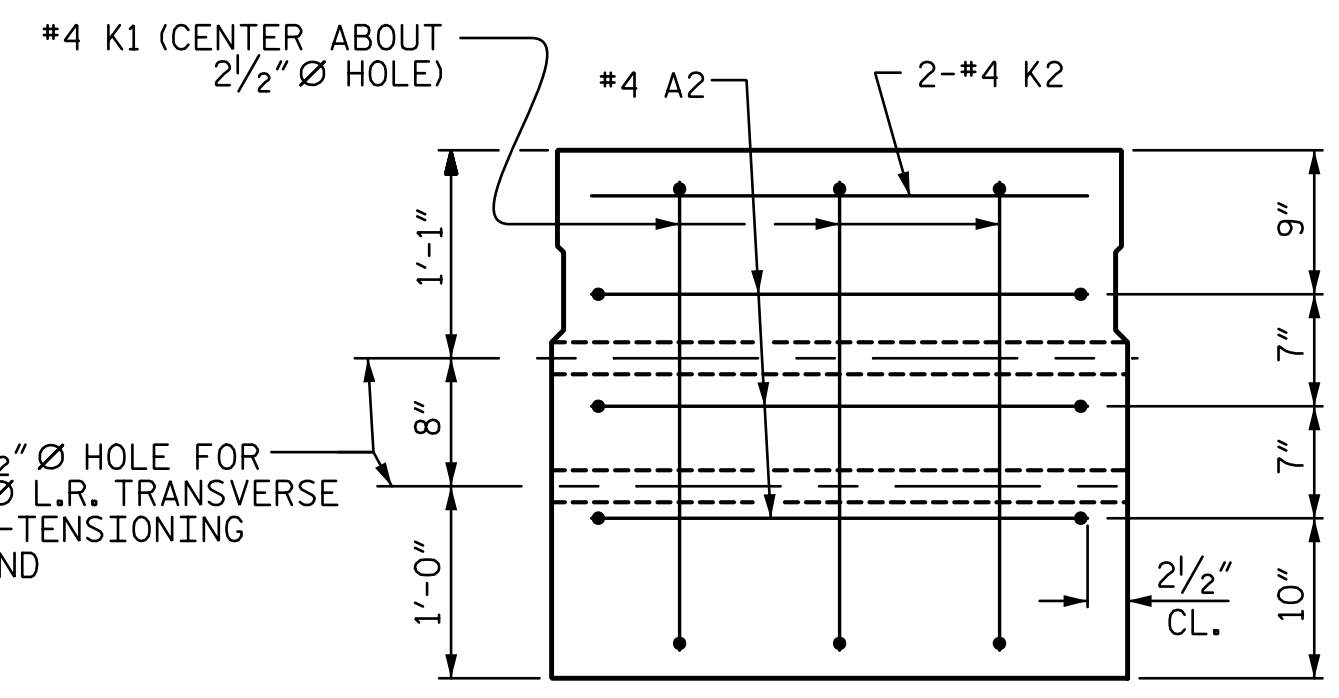
VIEW Y-Y
SHOWING ELEVATION VIEW OF GROUTED RECESS



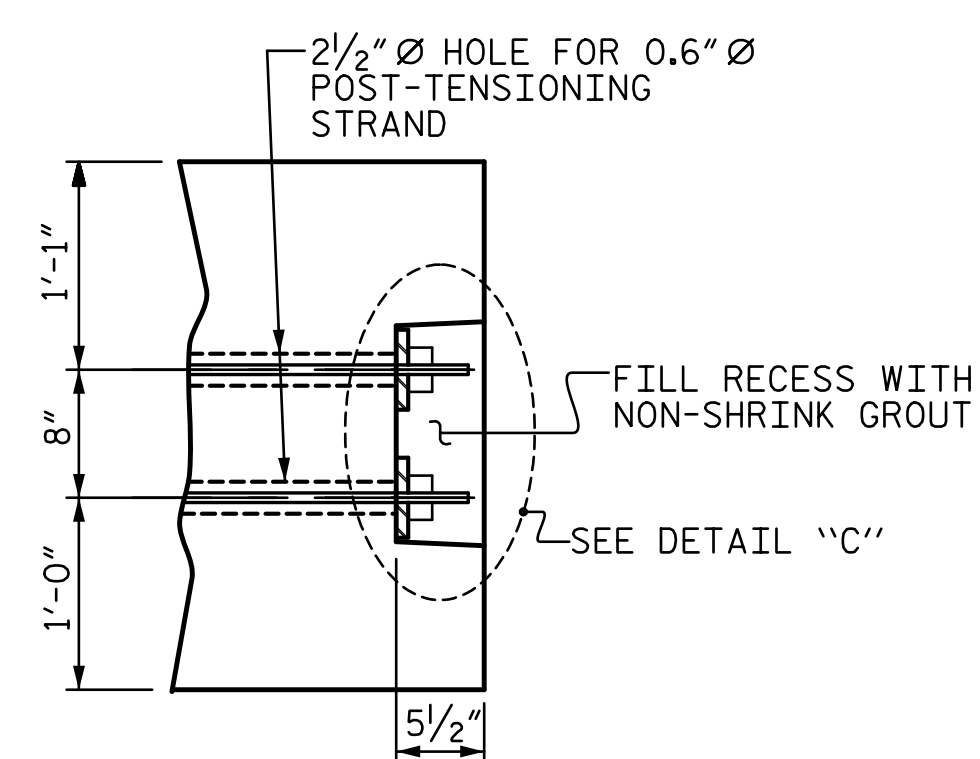
DETAIL "C"



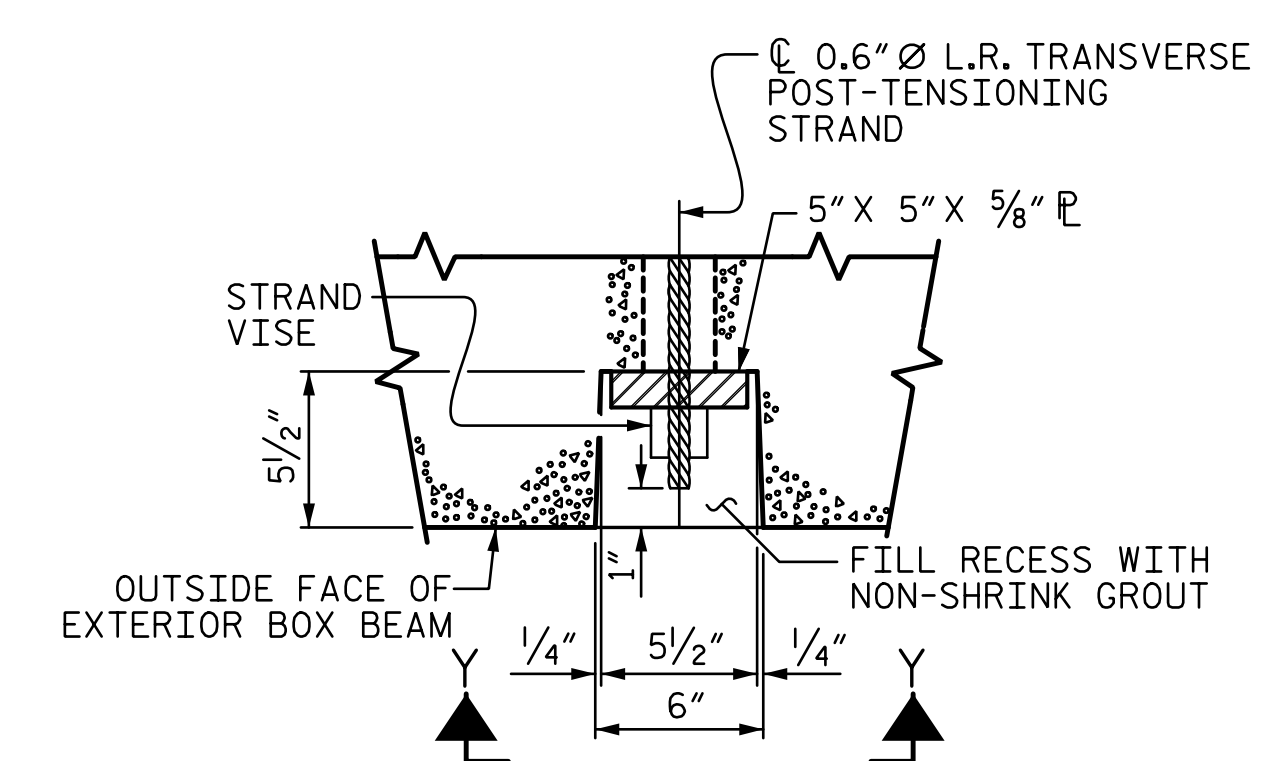
SECTION D-D



SECTION A-A
VOIDS NOT SHOWN



PART SECTION AT RECESS

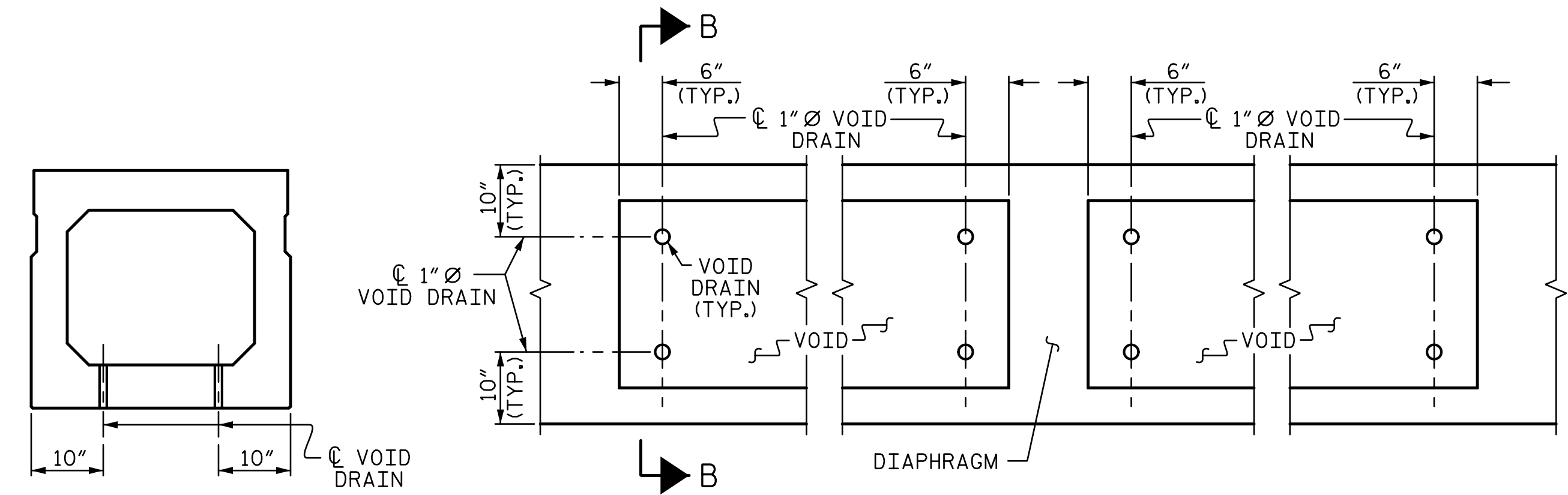


SECTION X-X
SHOWING PLAN VIEW OF GROUTED RECESS

DOUBLE DIAPHRAGM DETAILS

#4 "S" BARS NOT SHOWN. #4 "S" BARS MAY BE SHIFTED SLIGHTLY TO CLEAR 2 1/2" Ø HOLE.

GROUTED RECESS DETAIL AT END OF POST-TENSIONED STRANDS OF EXTERIOR BOX BEAM



SECTION B-B

PART PLAN

VOID DRAIN DETAILS
(DIMENSIONS SHOWN ARE TYPICAL FOR EACH VOID)

DEAD LOAD DEFLECTION AND CAMBER	
	3'-0" x 2'-9"
40' UNIT BOX BEAM UNIT (NC & SE)	0.6" Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	3/8" ↑
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	1/16" ↓
FINAL CAMBER	5/16" ↑

** INCLUDES FUTURE WEARING SURFACE

DEAD LOAD DEFLECTION AND CAMBER	
	3'-0" x 2'-9"
90' BOX BEAM UNIT (NC & SE)	0.6" Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	2 3/4" ↑
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	3/4" ↓
FINAL CAMBER	2" ↑

** INCLUDES FUTURE WEARING SURFACE

PROJECT NO. DF18314.2075090
POLK COUNTY
 STATION: 12+15.00 -L-

SHEET 6 OF 7

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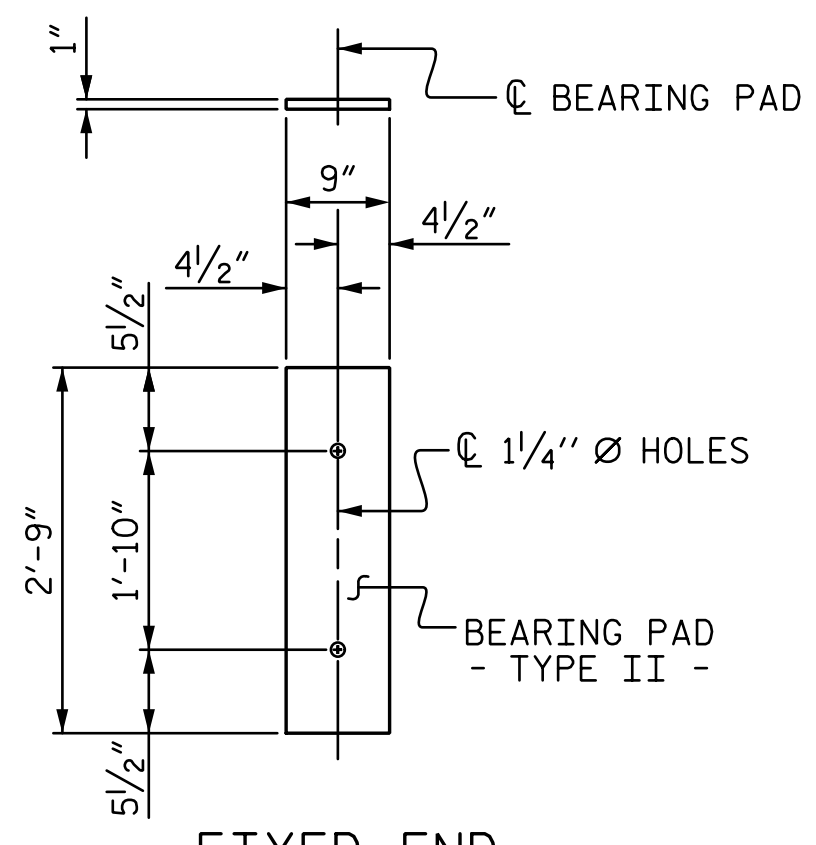
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

3'-0" X 2'-9"
PRESTRESSED CONCRETE
BOX BEAM UNIT

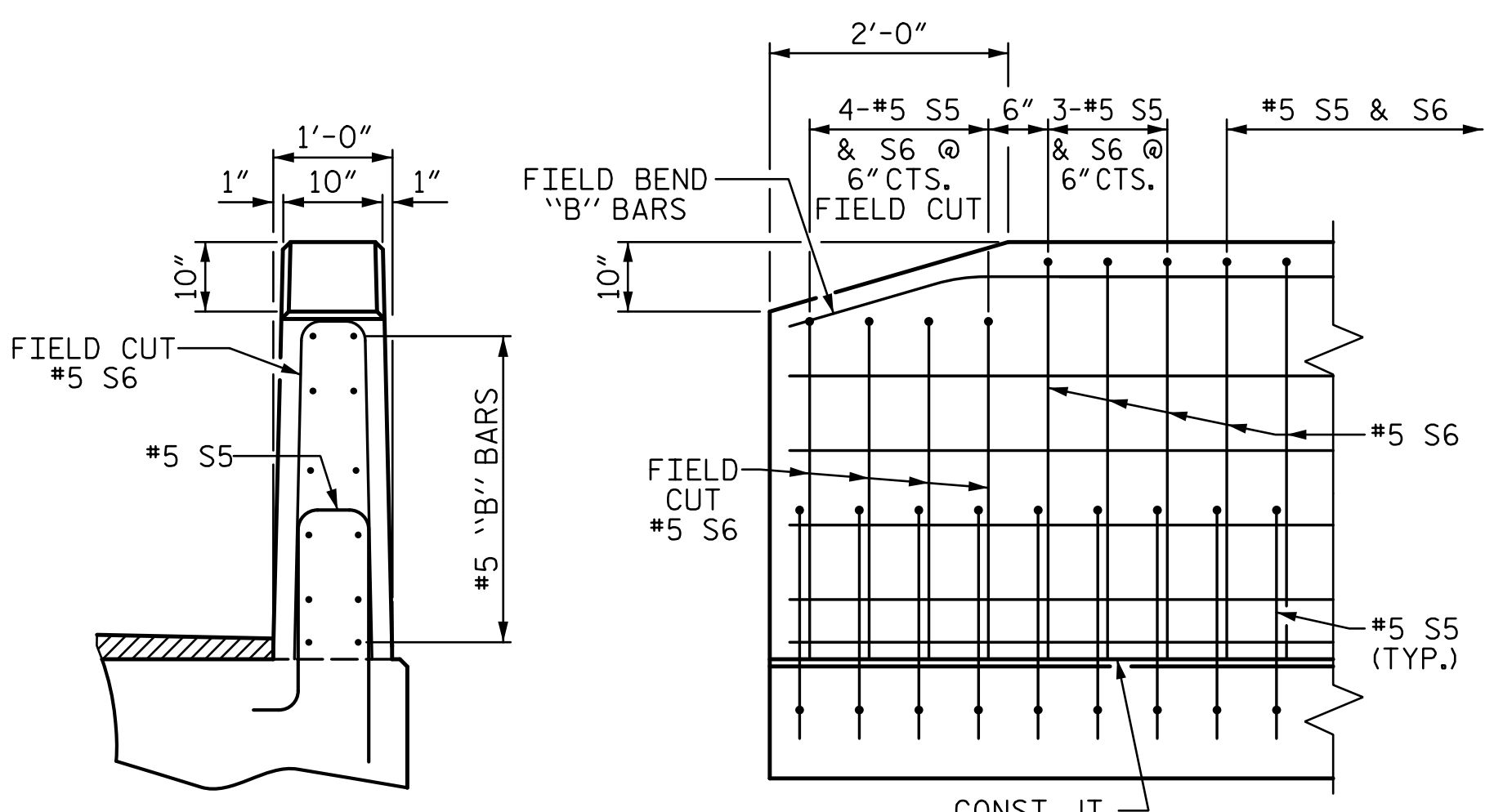
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NO.	BY:	DATE:	NO.	BY:	DATE:	S-12
1			3			TOTAL SHEETS 24
2			4			

DRAWN BY : T. R. LAWS DATE : 02/2025
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FIXED END
(TYPE II - 36 REQ'D)



END VIEW **SIDE VIEW**

END OF RAIL DETAILS

ELASTOMERIC BEARING DETAILS

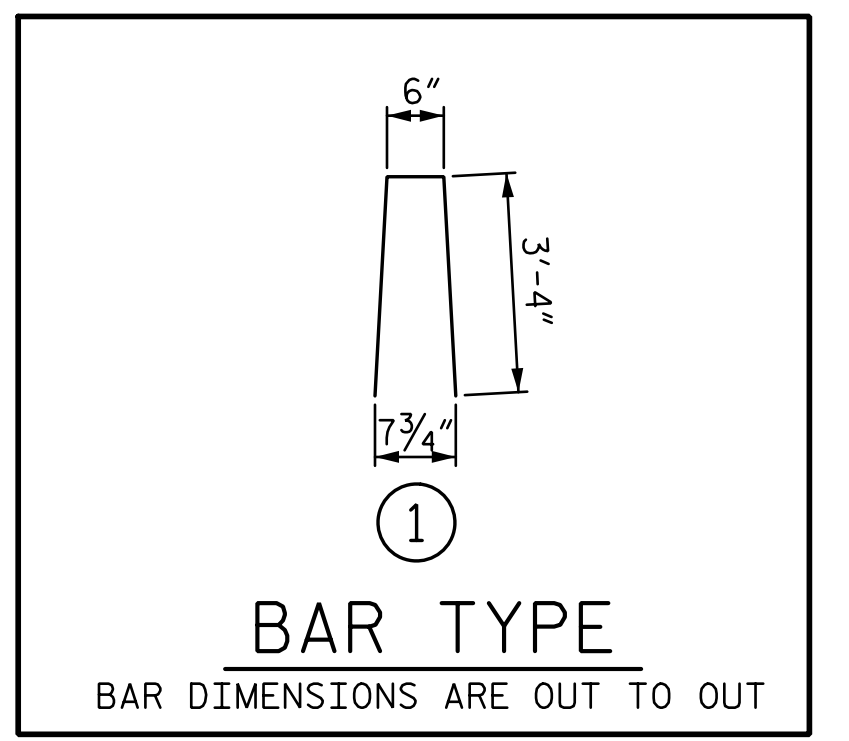
ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS.

BOX BEAM UNITS REQUIRED

	NUMBER	LENGTH	TOTAL LENGTH
EXTERIOR B.B.	2	40'-0"	80'-0"
INTERIOR B.B.	7	40'-0"	280'-0"
TOTAL	9		360'-0"

BOX BEAM UNITS REQUIRED

	NUMBER	LENGTH	TOTAL LENGTH
EXTERIOR B.B.	2	90'-0"	180'-0"
INTERIOR B.B.	7	90'-0"	630'-0"
TOTAL	9		810'-0"



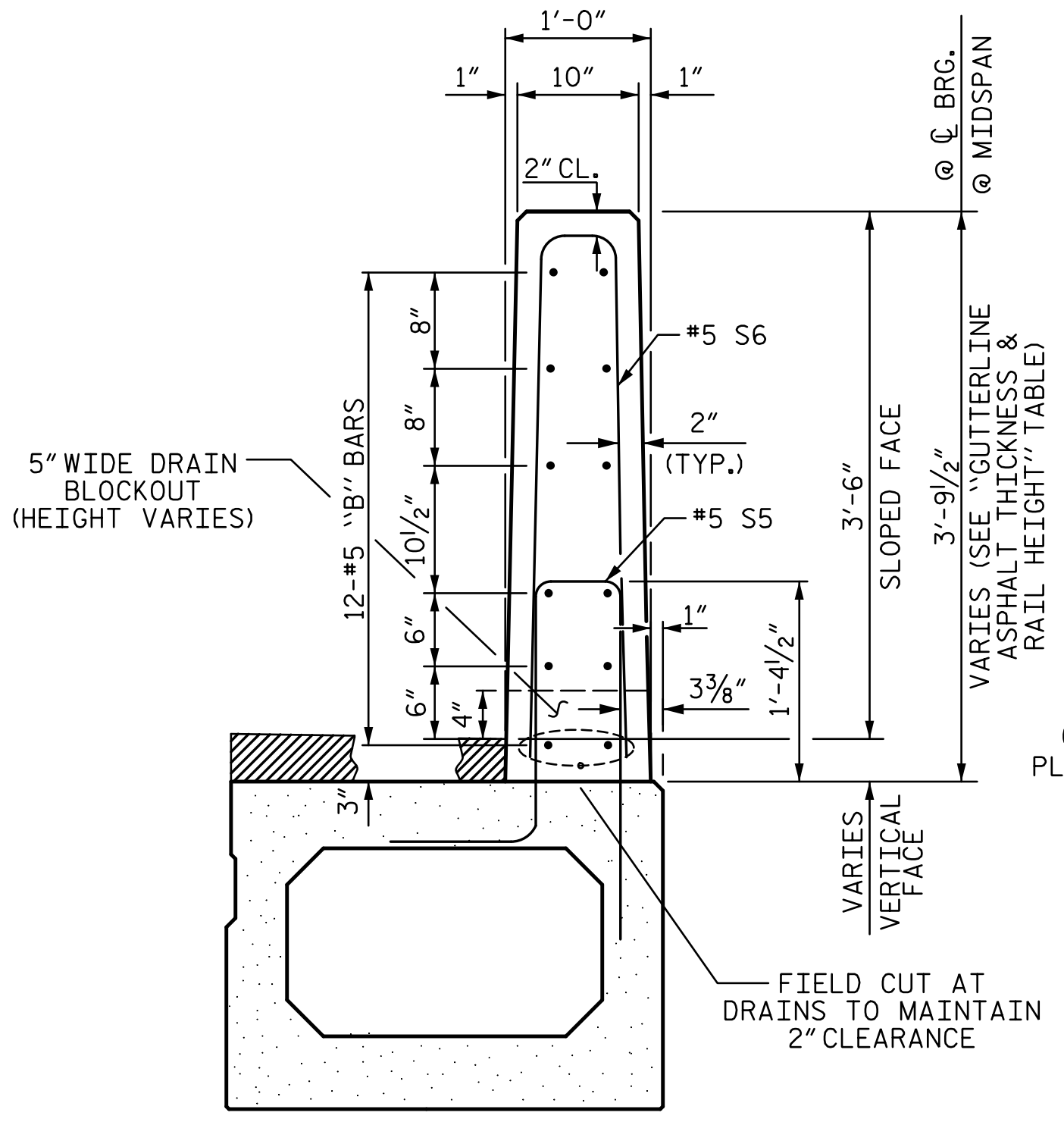
BAR TYPE
BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL

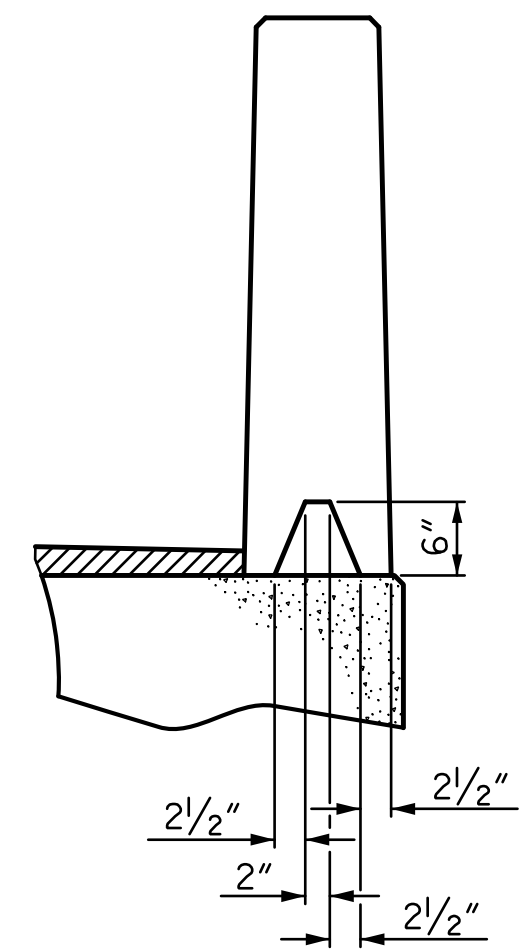
BAR	BARS PER PAIR OF EXTERIOR UNITS	SIZE	TYPE	LENGTH	WEIGHT
40' UNIT					
* B7	48	#5	STR	19'-8"	985
* S6	126	#5	1	7'-2"	942
* EPOXY COATED REINFORCING STEEL				LBS.	1927
CLASS AA CONCRETE				CU.YDS.	10.4
TOTAL VERTICAL CONCRETE BARRIER RAIL				LN. FT.	80.0

BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL

BAR	BARS PER PAIR OF EXTERIOR UNITS	SIZE	TYPE	LENGTH	WEIGHT
90' UNIT					
* B10	96	#5	STR	22'-1"	2211
* S6	252	#5	1	7'-2"	1884
* EPOXY COATED REINFORCING STEEL				LBS.	4095
CLASS AA CONCRETE				CU.YDS.	23.3
TOTAL VERTICAL CONCRETE BARRIER RAIL				LN. FT.	180.0

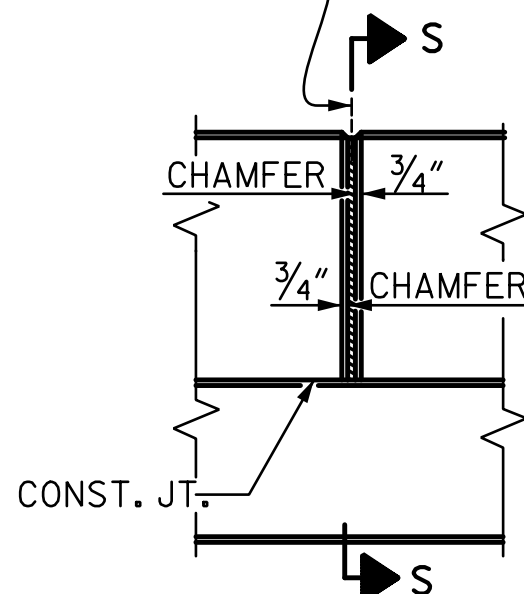


SECTION THRU RAIL



SECTION S-S
AT DAM IN OPEN JOINT
(THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)

1/2" EXP. JT. MAT'L HELD IN PLACE WITH GALVANIZED NAILS.
(NOTE: OMIT EXP. JT. MAT'L WHEN SLIP FORM IS USED)



ELEVATION AT EXPANSION JOINTS

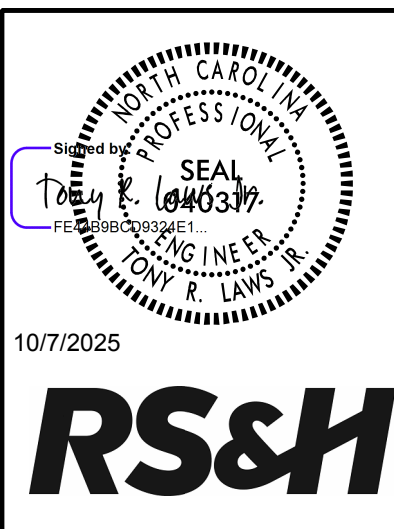
GUTTERLINE ASPHALT THICKNESS & RAIL HEIGHT

	ASPHALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN
40' UNITS	3 3/16"	3'-9 3/16"
90' UNITS	1 1/2"	3'-7 1/2"

VERTICAL CONCRETE BARRIER RAIL DETAILS

PROJECT NO. DF18314.2075090
POLK COUNTY
STATION: 12+15.00 -L-

SHEET 7 OF 7



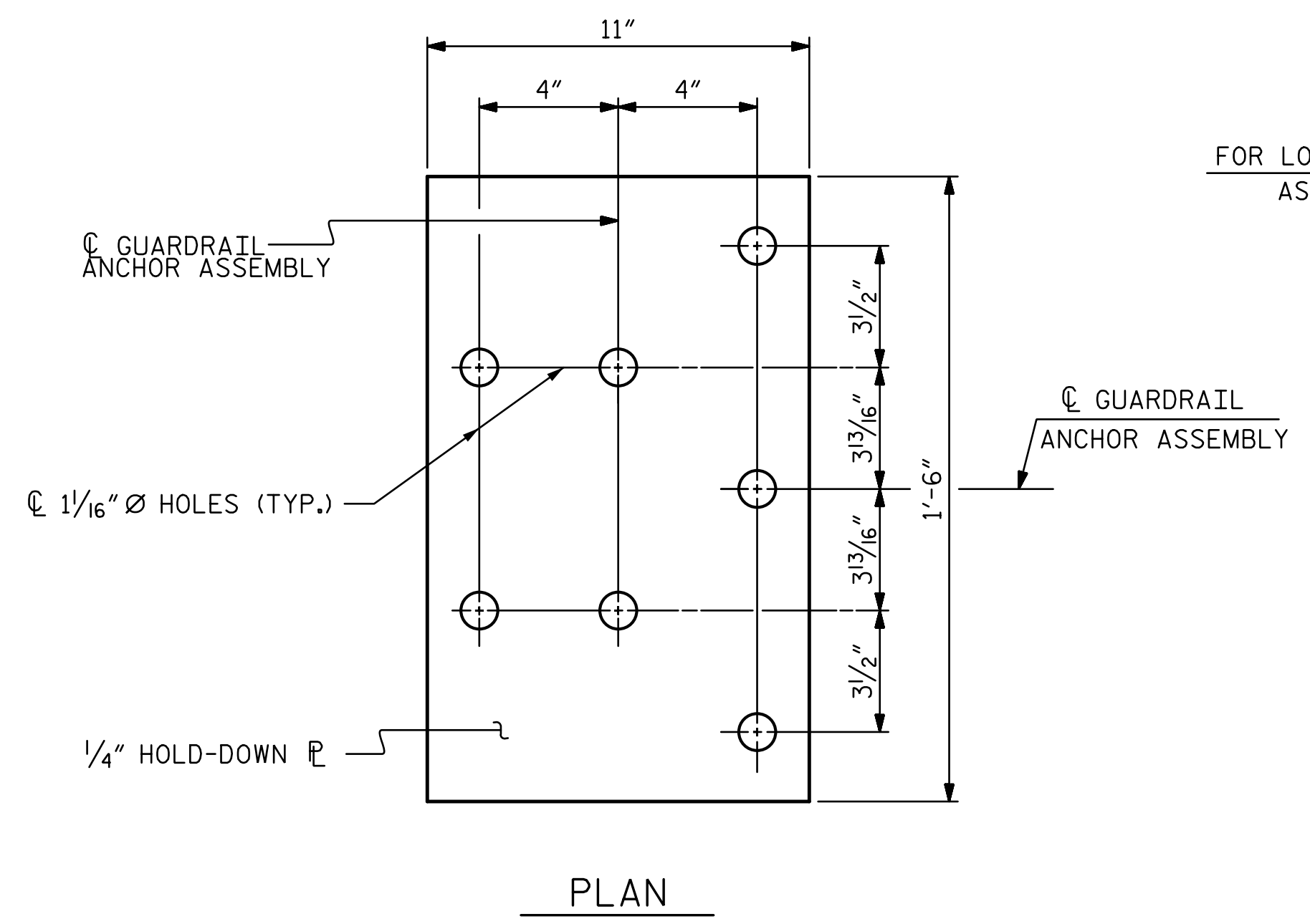
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

3'-0" X 2'-9"
PRESTRESSED CONCRETE
BOX BEAM UNIT

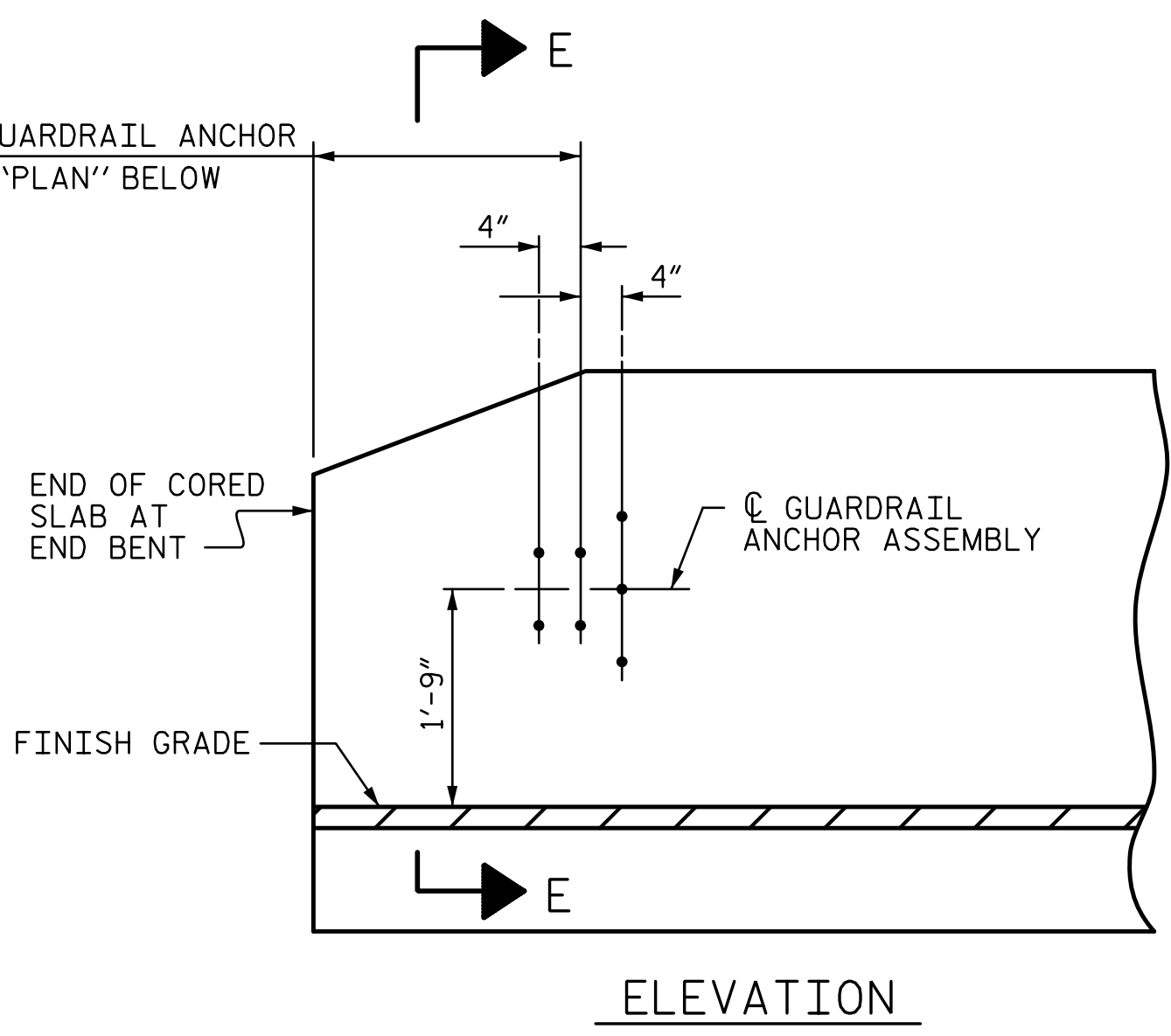
DRAWN BY : T. R. LAWS DATE : 02/2025
CHECKED BY : V. SELLERS DATE : 02/2025
DESIGN ENGINEER OF RECORD: T. R. LAWS DATE : 10/2025

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1			3			TOTAL SHEETS
2			4			24

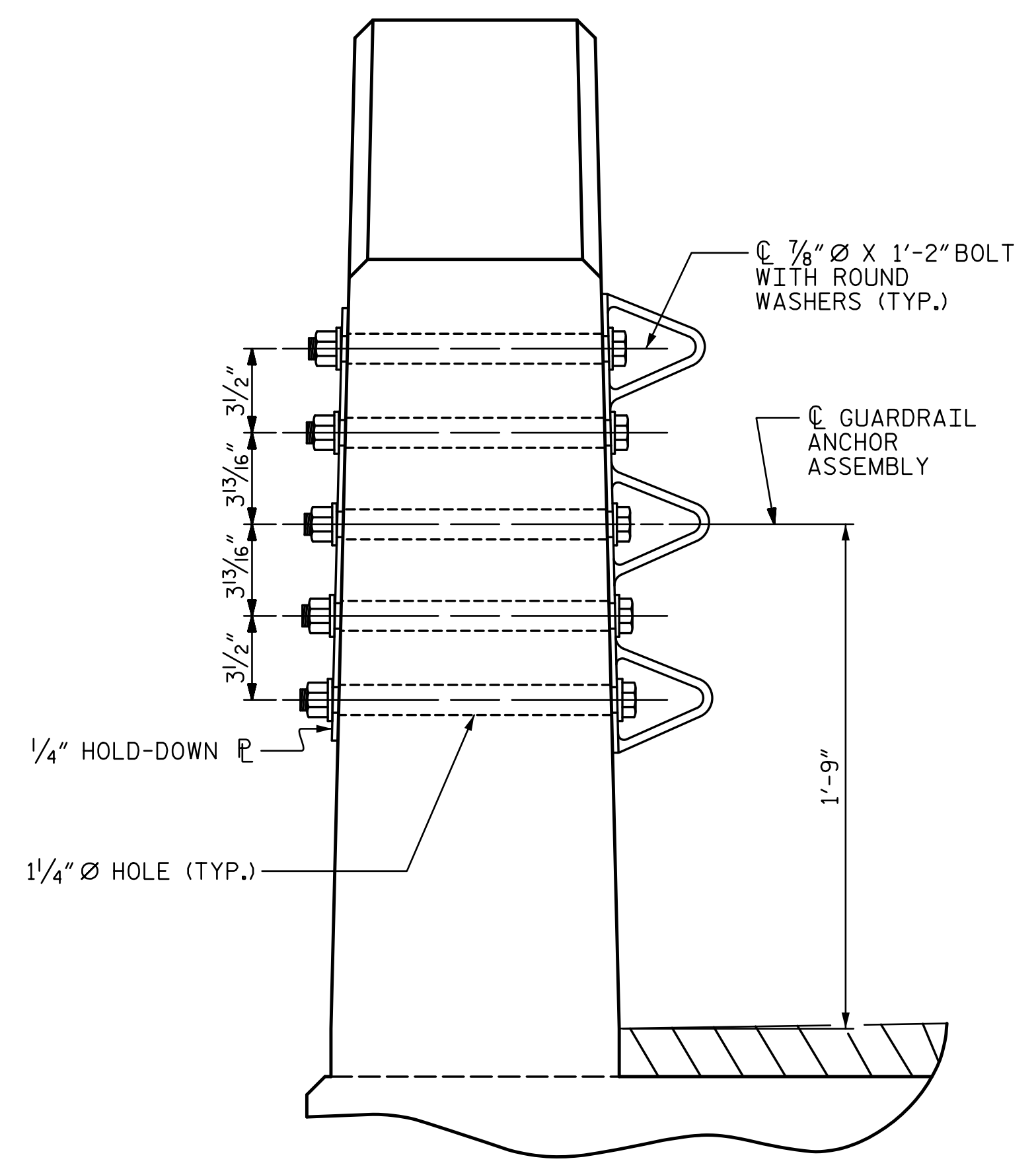


FOR LOCATION OF GUARDRAIL ANCHOR ASSEMBLY, SEE "PLAN" BELOW

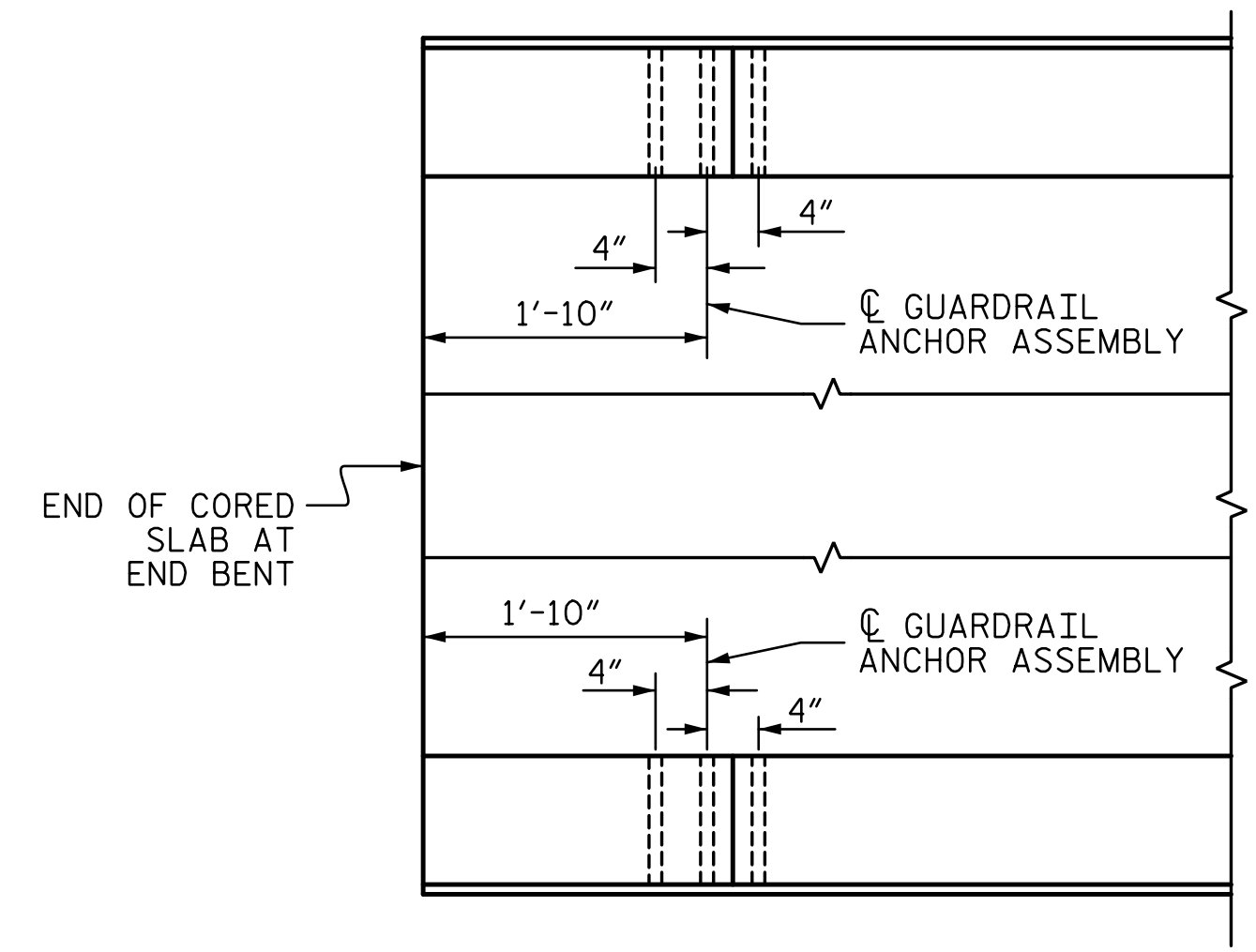


PLAN

ELEVATION



SECTION E-E
GUARDRAIL ANCHOR ASSEMBLY DETAILS



LOCATION OF ANCHORS FOR GUARDRAIL

END BENT 1 SHOWN, END BENT 2 SIMILAR.

PLAN



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

NOTES

- THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 7 - 7/8" Ø BOLTS WITH NUTS AND WASHERS.
- THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.
- BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
- THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.
- AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.
- THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.
- THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.
- THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

PROJECT NO. DF18314.2075090

 POLK COUNTY
 STATION: 12+15.00 -L-

ASSEMBLED BY :	T. R. LAWS	DATE :	02/2025
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DRAWN BY :	MAA 5/10	REV. 1/15	MAA/TMG
CHECKED BY :	GM 5/10	REV. 12/17	MAA/TMG
		REV. 5/18	MAA/TMG

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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD GUARDRAIL ANCHORAGE DETAILS FOR VERTICAL CONCRETE BARRIER RAIL					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO.					S-14
TOTAL SHEETS					24

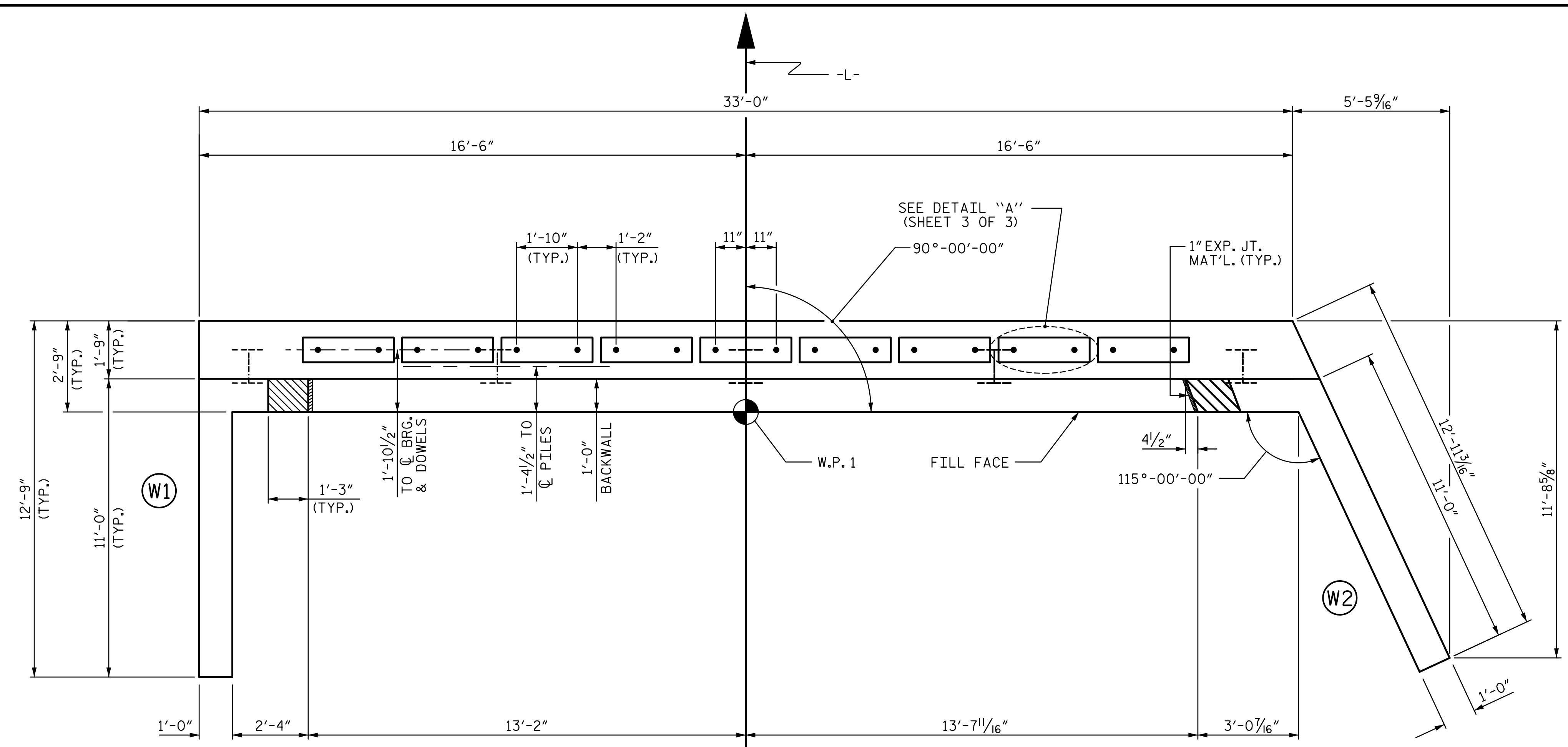
NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

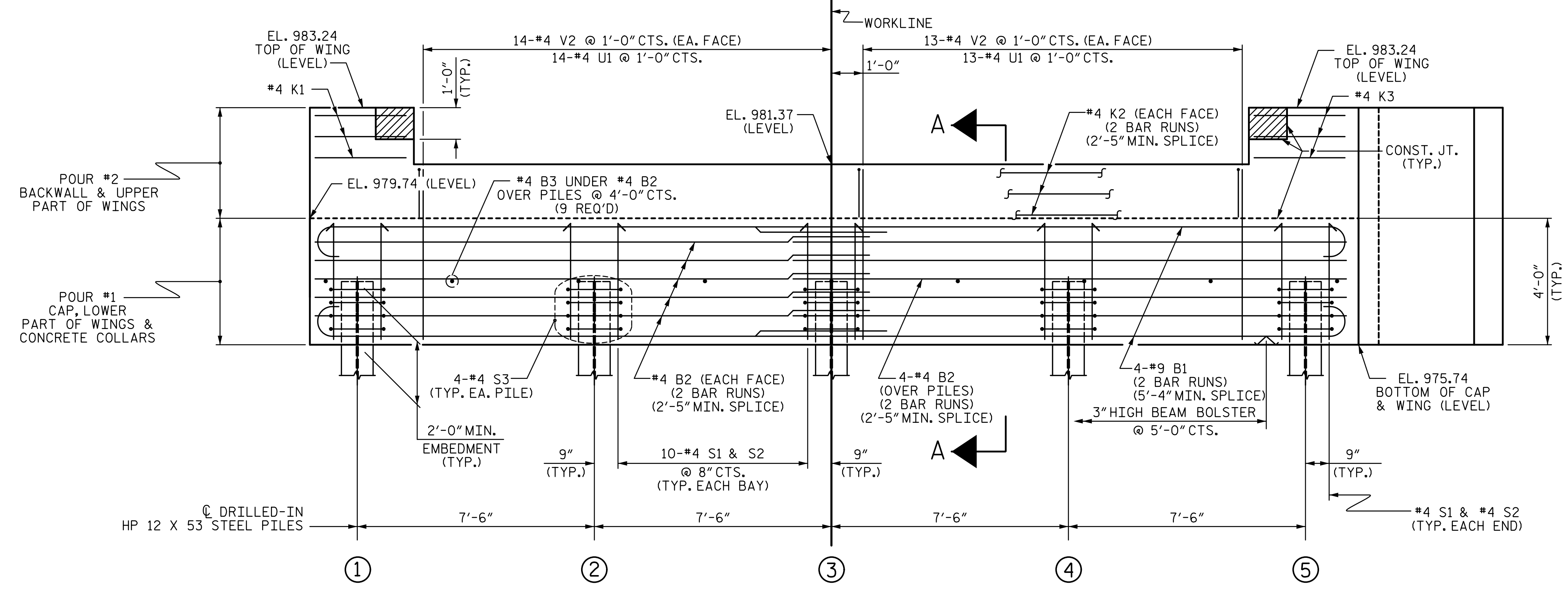
THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR WING DETAILS, SEE SHEET 2 OF 3.

FOR PILE SPLICE DETAILS, SEE SHEET 3 OF 3.



PLAN

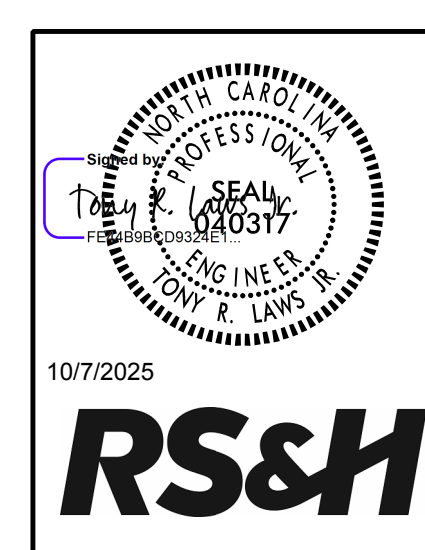


ELEVATION

FOR SECTION A-A, SEE SHEET 3 OF 3.
 CONCRETE COLLARS FOR STEEL PILES NOT SHOWN IN PLAN AND ELEVATION VIEWS FOR CLARITY.
 SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL", SHEET 3 OF 3.

PROJECT NO. DF18314.2075090
POLK COUNTY
 STATION: 12+15.00 -L-

SHEET 1 OF 3

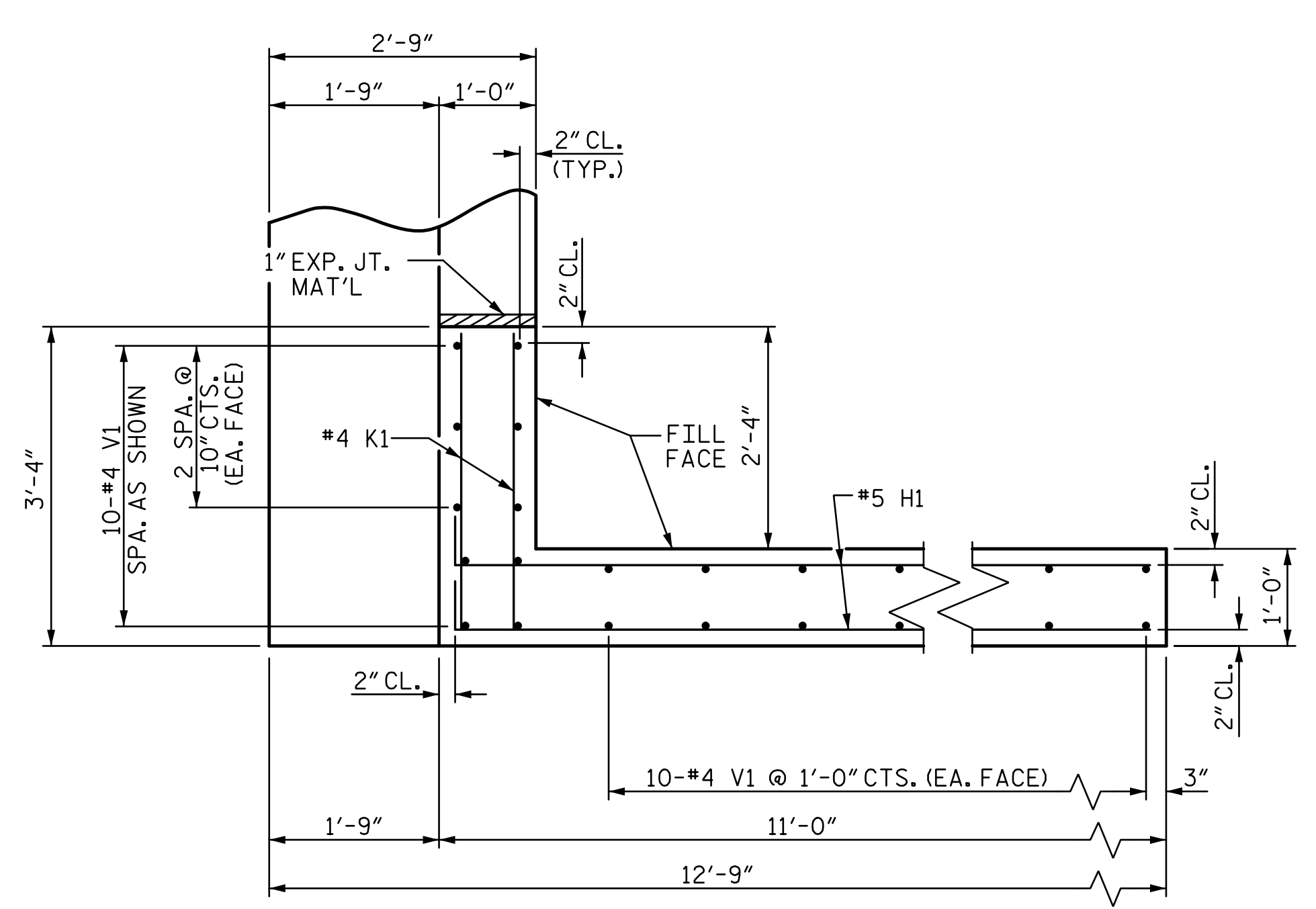


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 END BENT 1

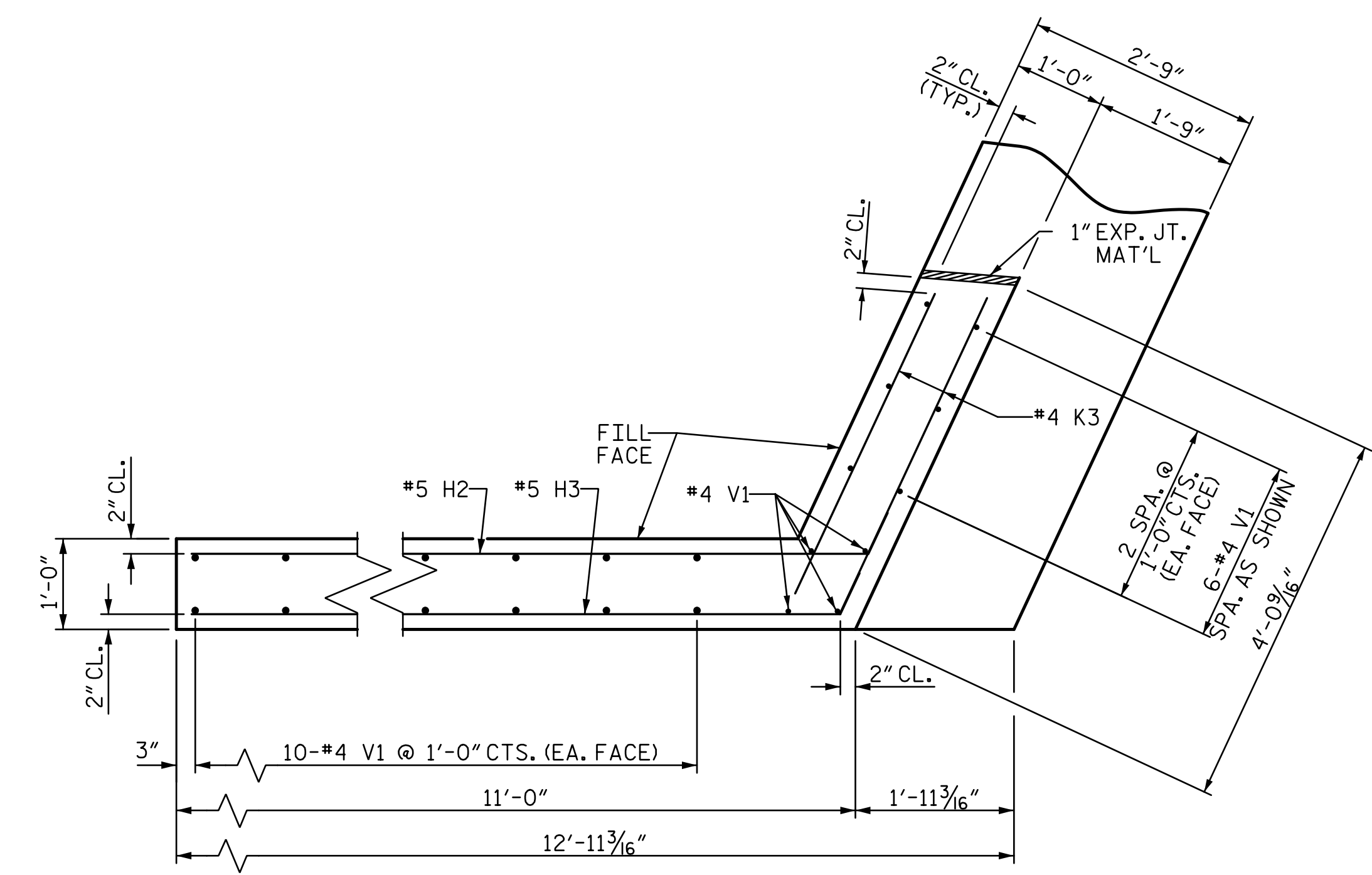
DRAWN BY : J. SCACCA DATE : 05/2025
 CHECKED BY : M. ACOSTA DATE : 07/2025
 DESIGN ENGINEER OF RECORD: T. R. LAWS DATE : 10/2025

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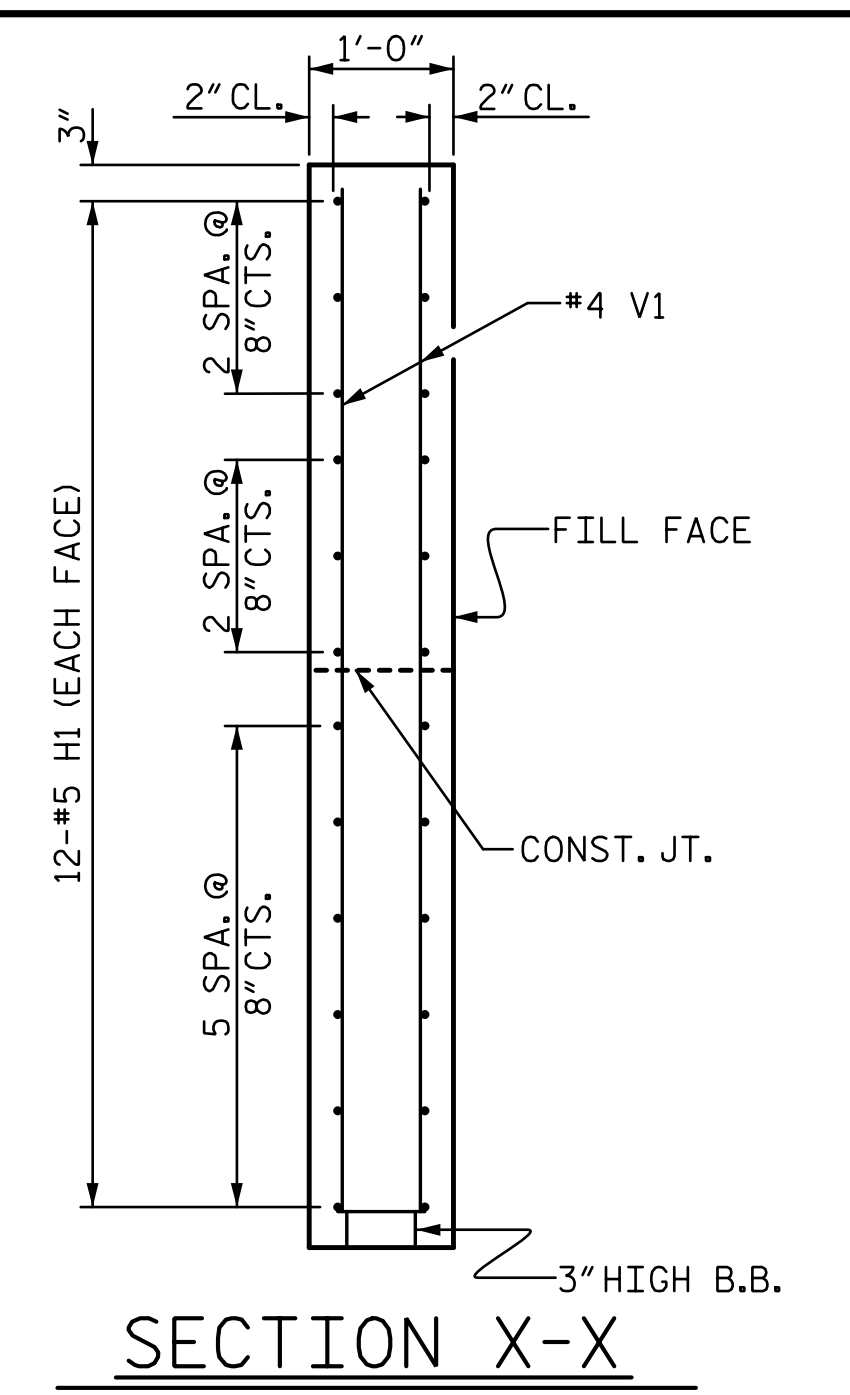
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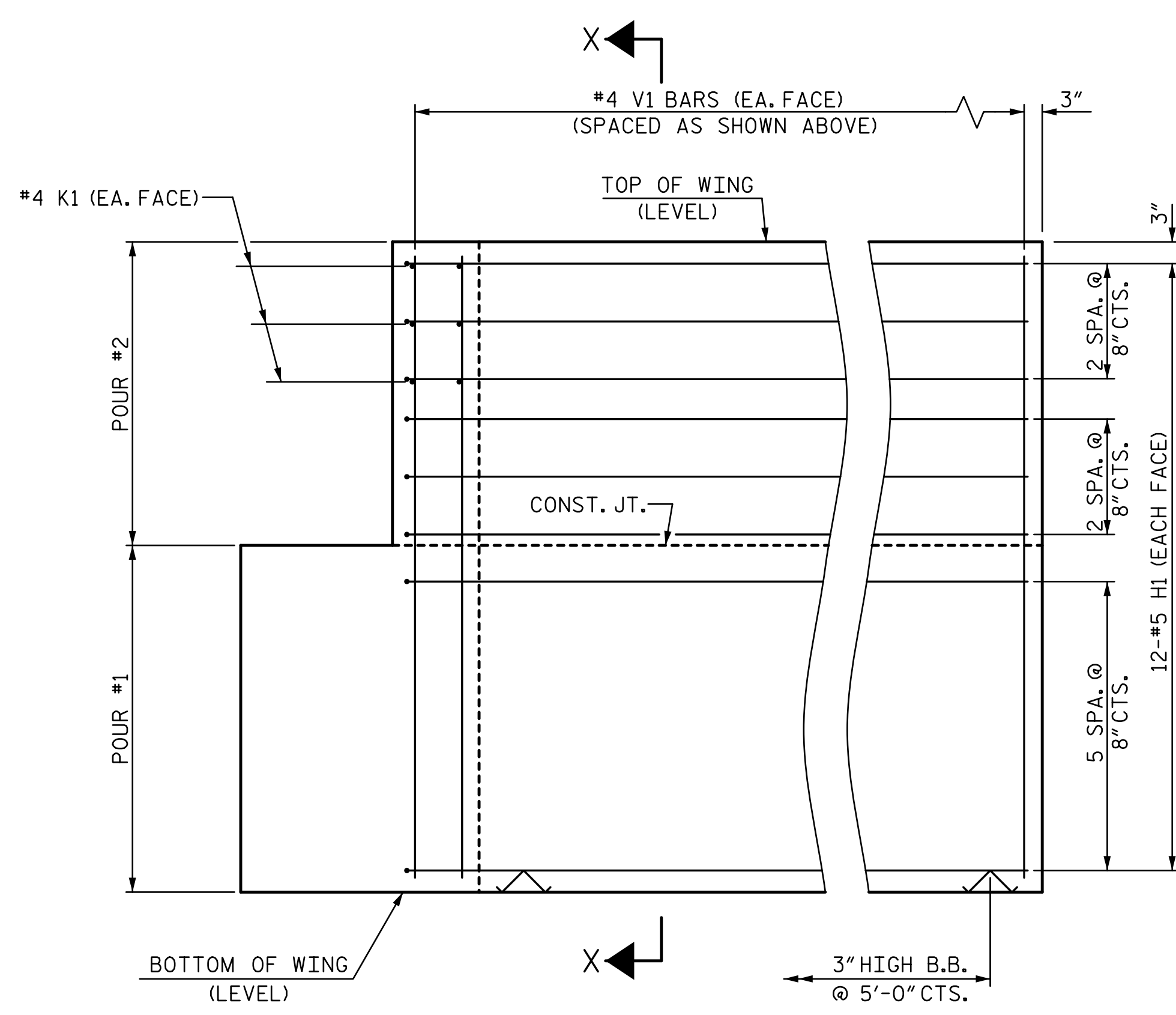
PLAN OF WING (W1)



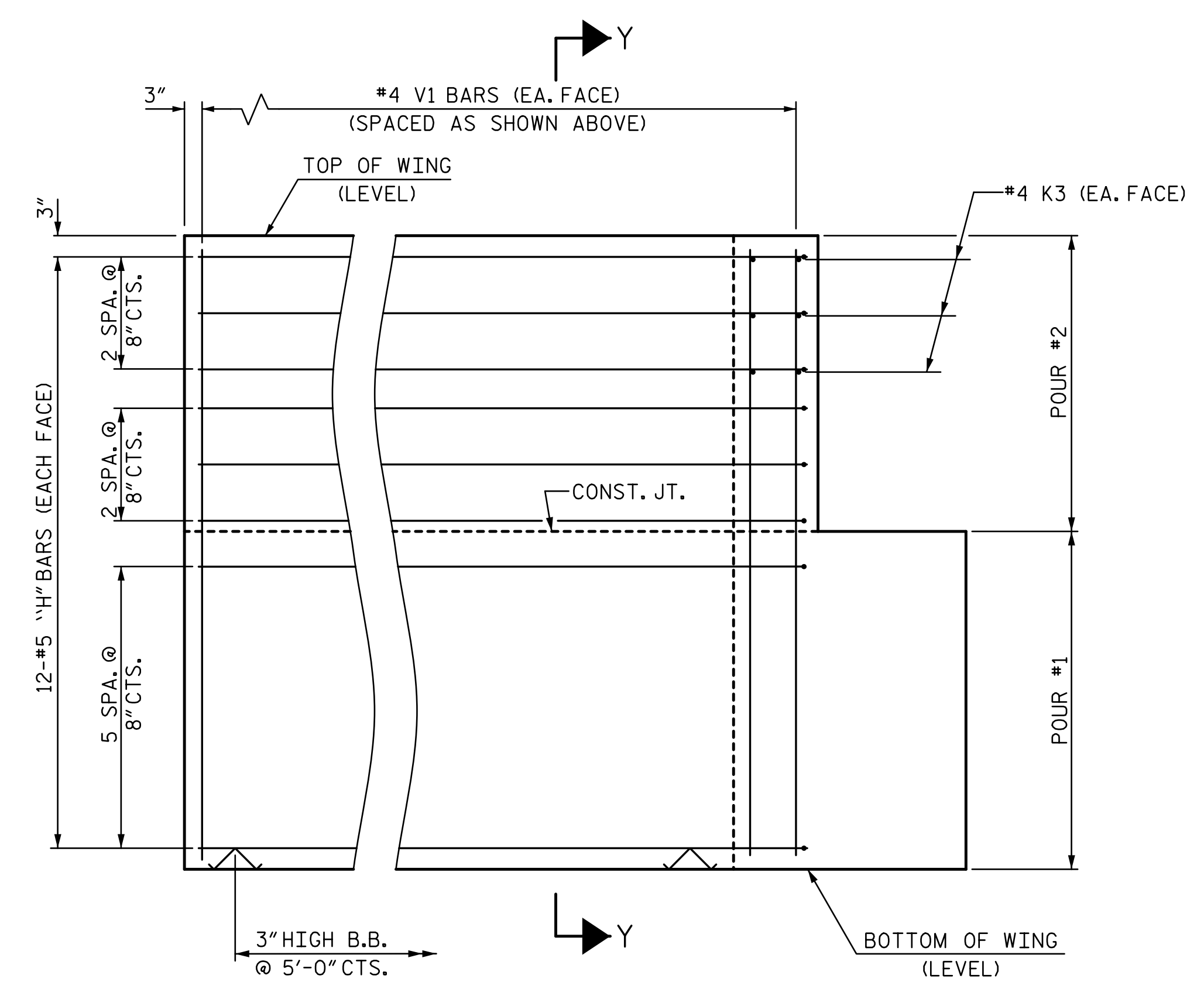
PLAN OF WING (W2)



SECTION X-X

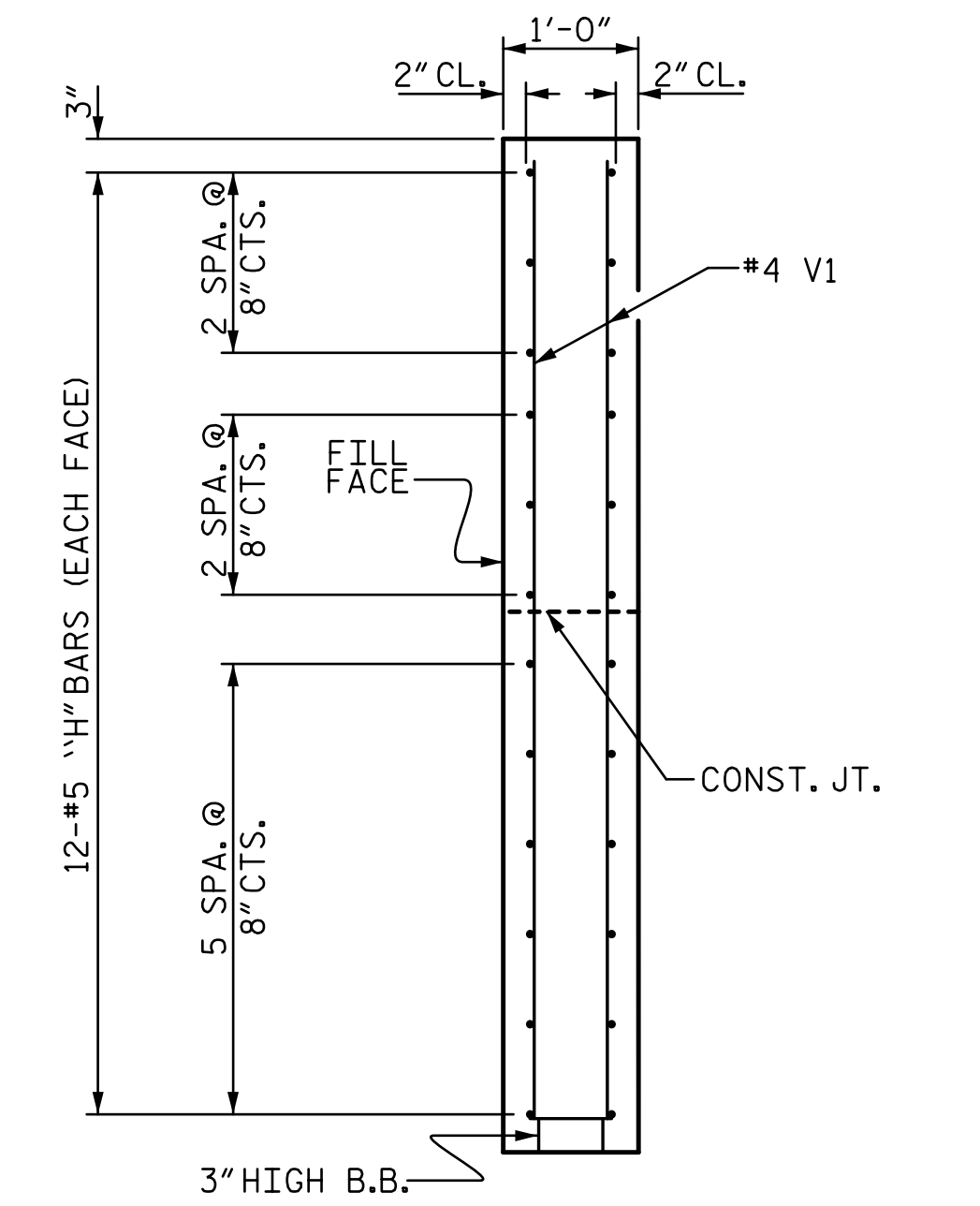


ELEVATION OF WING (W1)



ELEVATION OF WING (W2)

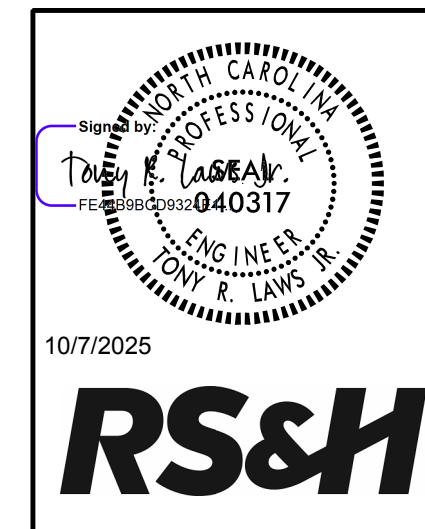
WING DETAILS



SECTION Y-Y

PROJECT NO. DF18314.2075090
POLK COUNTY
 STATION: 12+15.00 -L-

SHEET 2 OF 3



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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 END BENT 1
 WING DETAILS

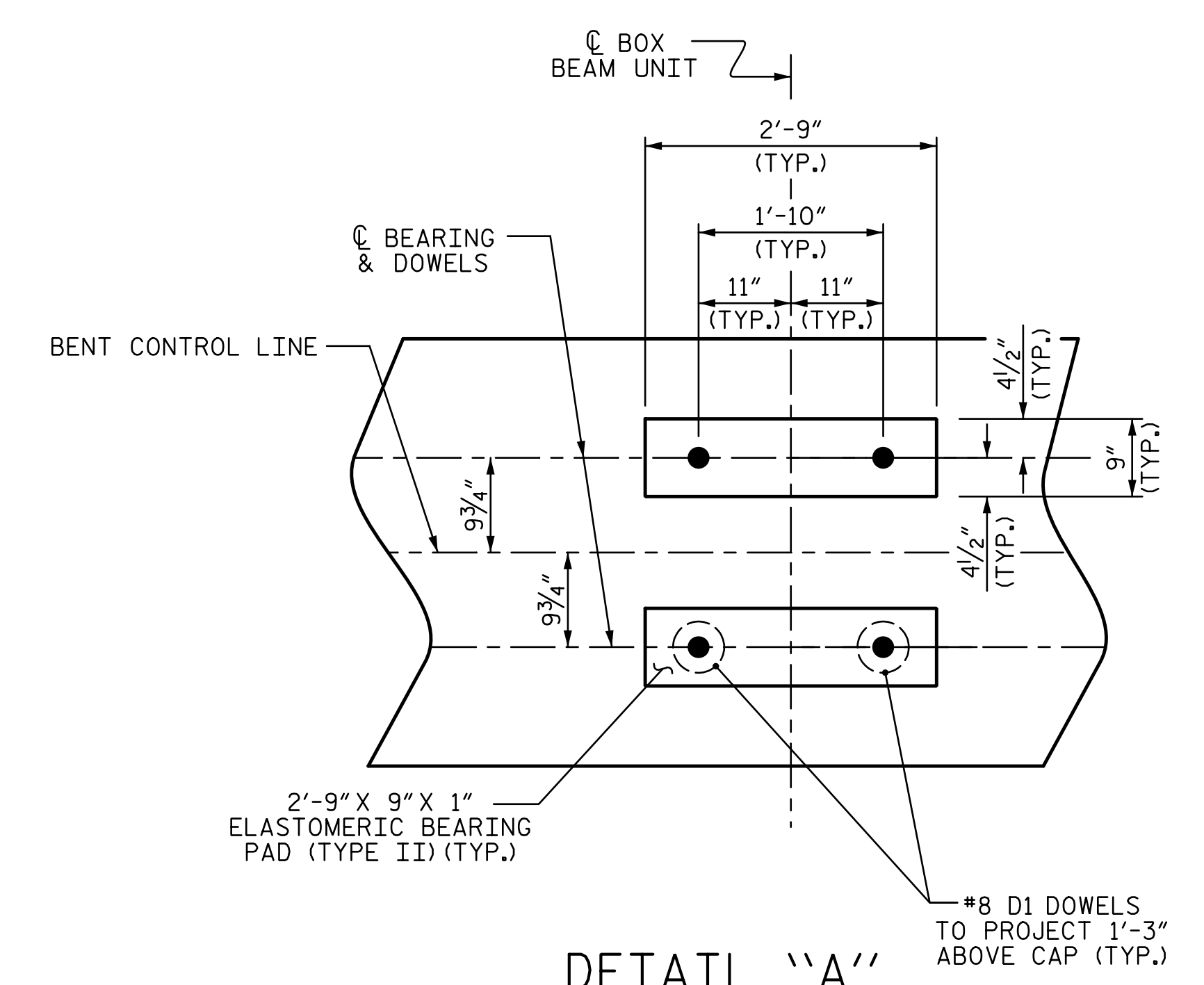
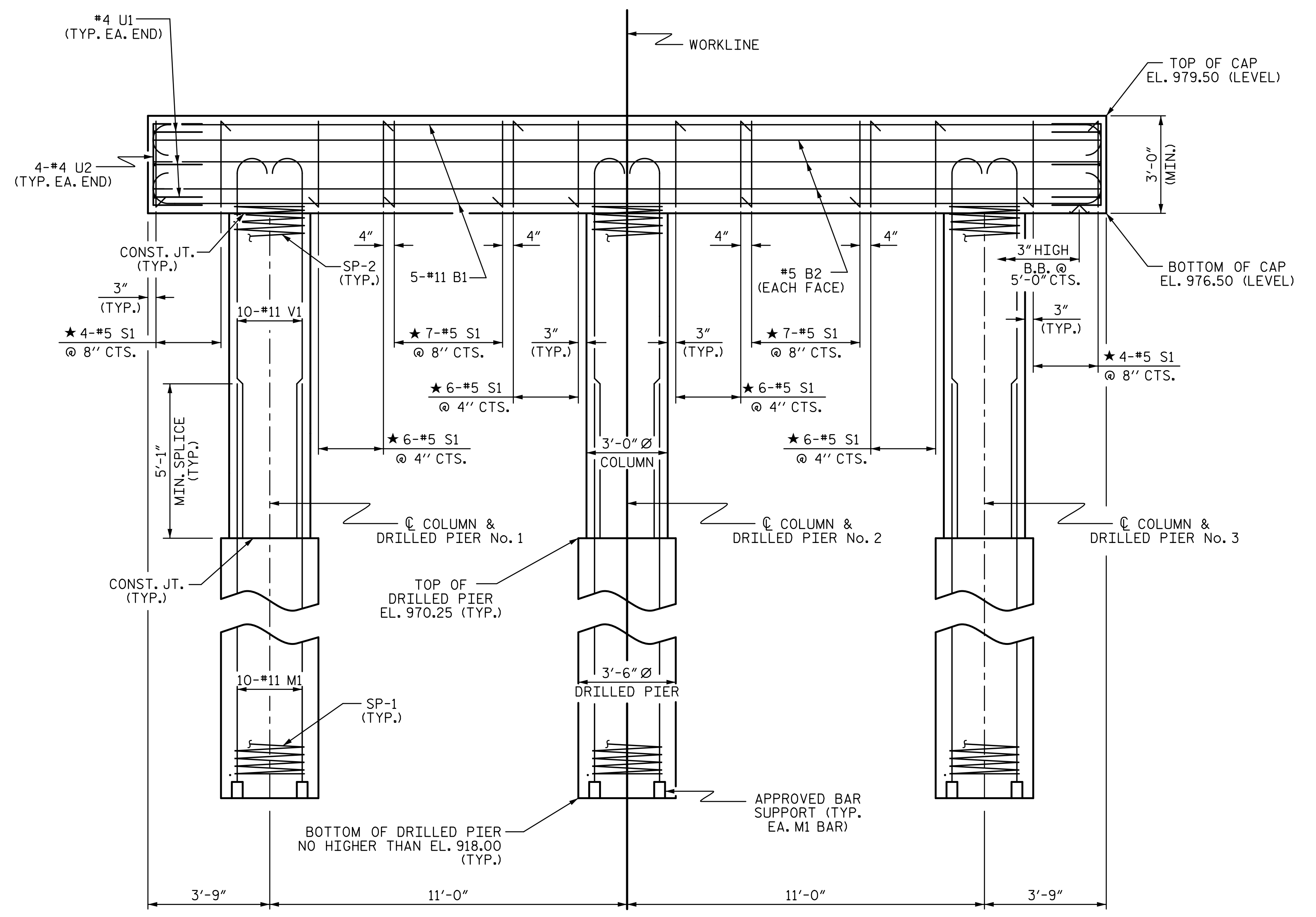
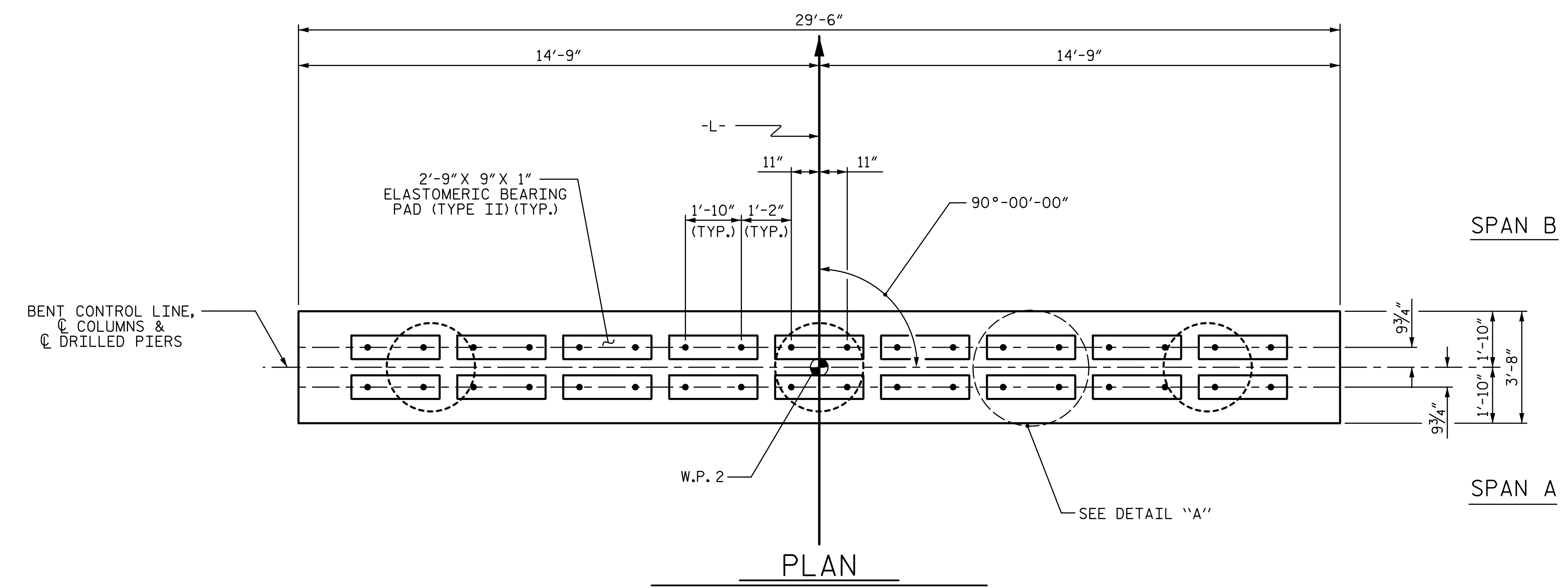
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1			3			TOTAL SHEETS
2			4			24

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DRAWN BY : J. SCACCA DATE : 05/2025
 CHECKED BY : M. ACOSTA DATE : 07/2025
 DESIGN ENGINEER OF RECORD: T. R. LAWS DATE : 10/2025

NOTES

- STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.
- HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.
- FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.
- ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL" AND "SPIRAL COLUMN REINFORCING STEEL."
- ★ INVERT ALTERNATE STIRRUPS.
- THE LOCATION OF THE CONSTRUCTION JOINT IN THE DRILLED PIERS IS BASED ON AN APPROXIMATE GROUND LINE ELEVATION. IF THE CONSTRUCTION JOINT IS ABOVE THE ACTUAL GROUND LINE ELEVATION, THE CONTRACTOR SHALL PLACE THE CONSTRUCTION JOINT 1 FT. BELOW THE GROUND LINE.
- THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.



DETAIL "A"
 (DIMENSIONS ARE TYPICAL EACH BEARING)
 PROJECT NO. DF18314.2075090
 POLK COUNTY
 STATION: 12+15.00 -L-

SHEET 1 OF 2

10/7/2025
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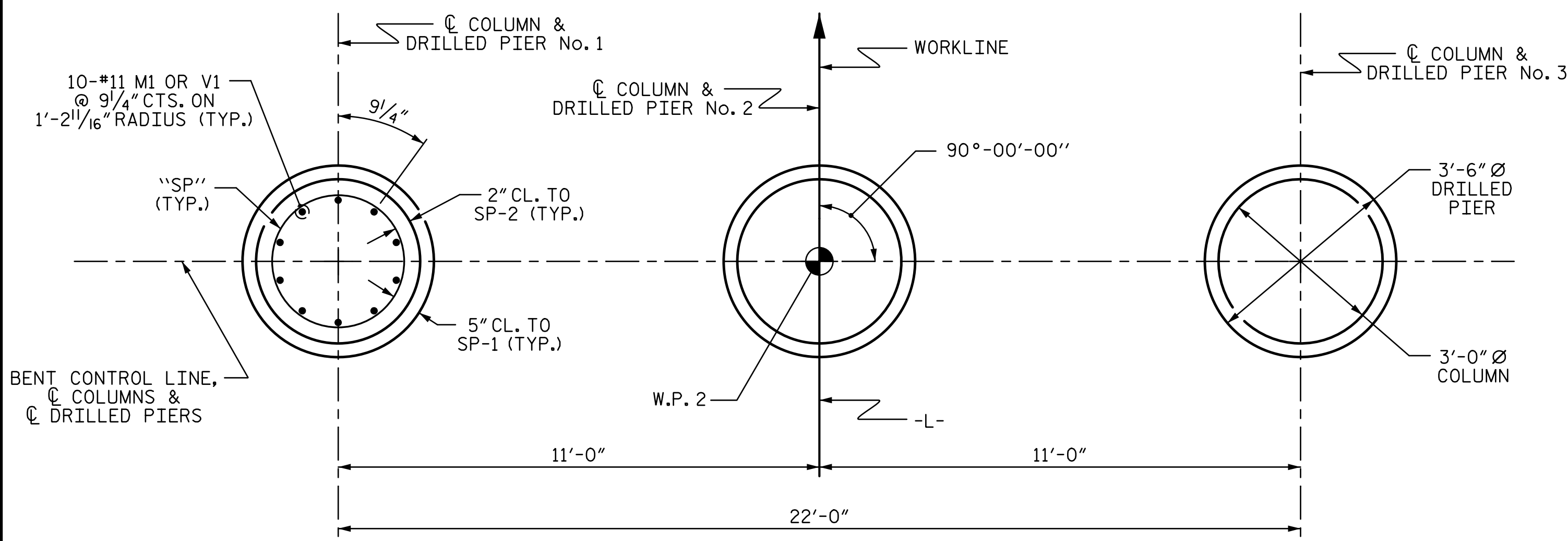
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
BENT 1

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-18
1			3			TOTAL SHEETS 24
2			4			

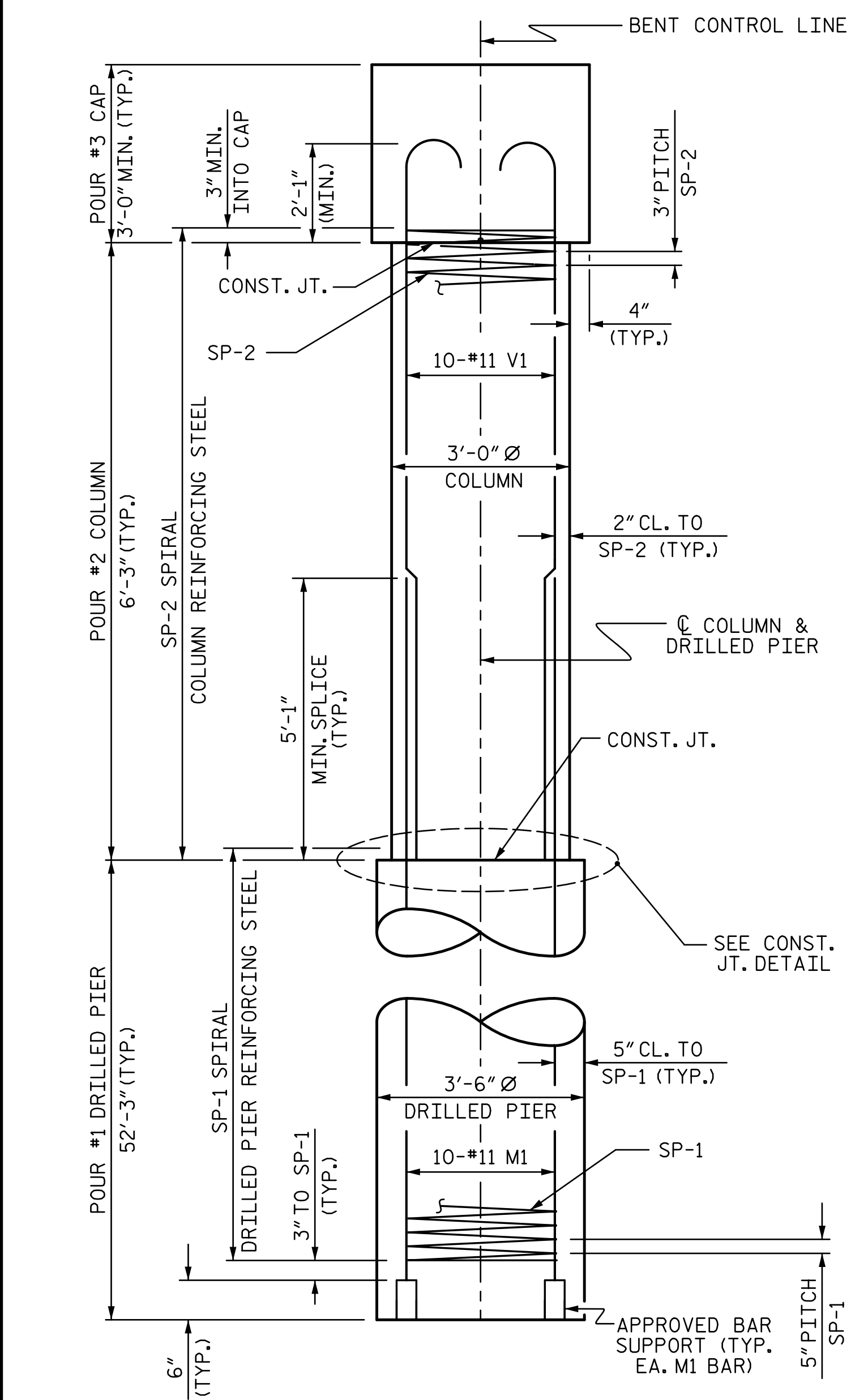
DRAWN BY : J. SCACCA DATE : 02/2025
 CHECKED BY : T. R. LAWS DATE : 02/2025
 DESIGN ENGINEER OF RECORD: T. R. LAWS DATE : 10/2025

DIMENSIONS & REINFORCING STEEL ARE TYPICAL FOR EACH COLUMN & DRILLED PIER UNLESS OTHERWISE NOTED.

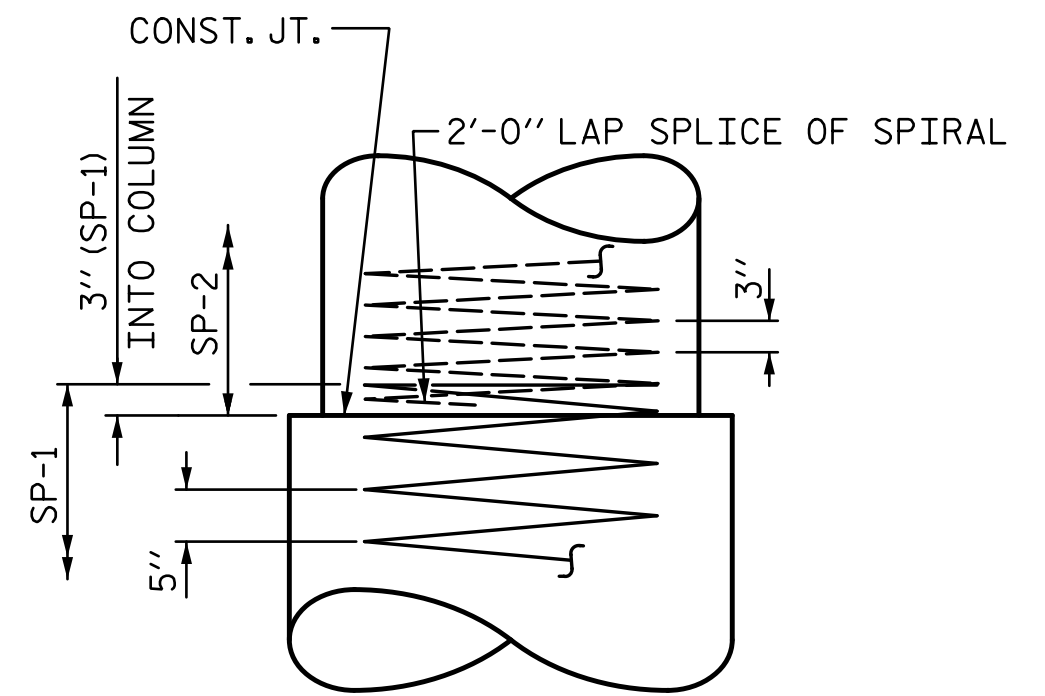
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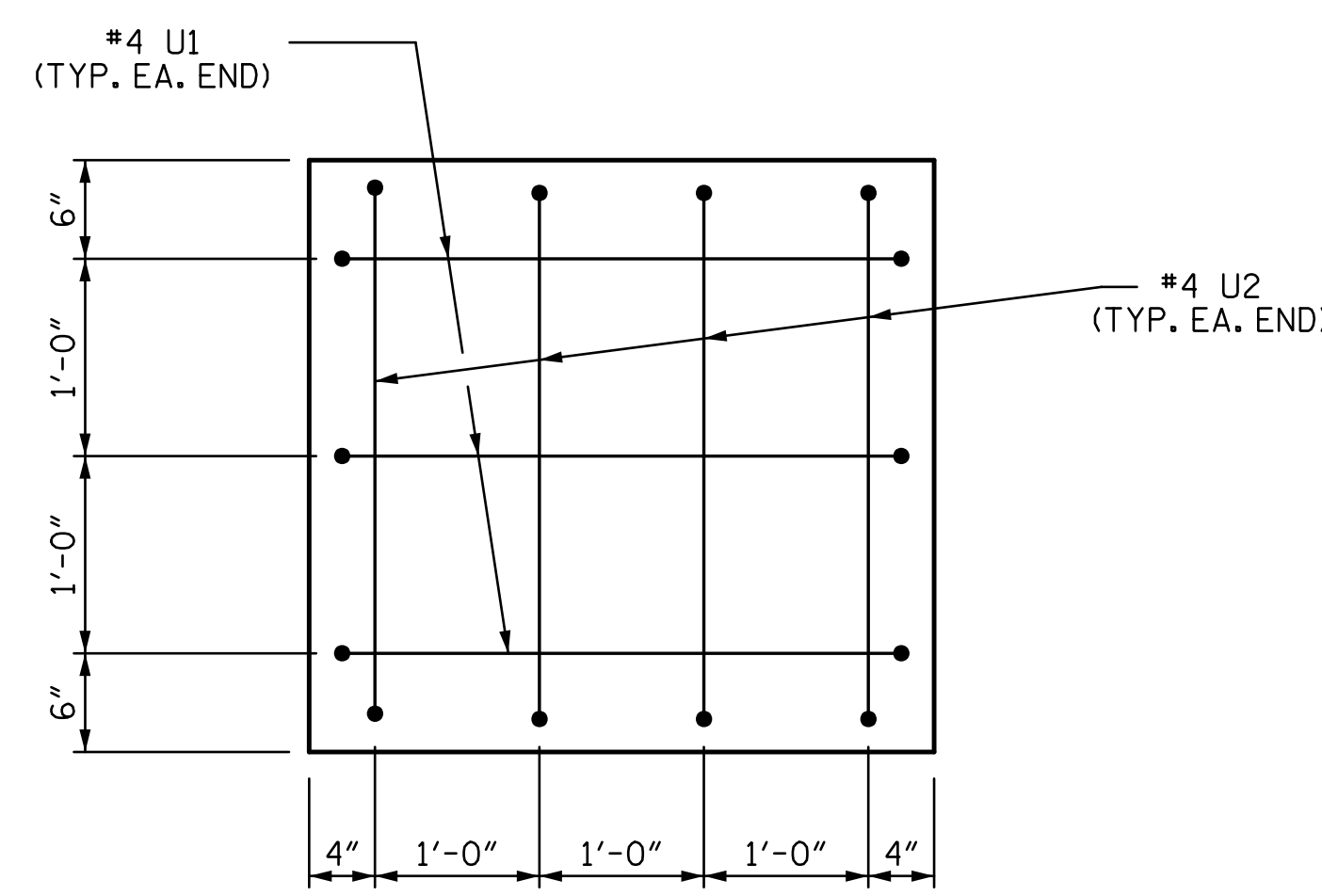
PLAN OF DRILLED PIERS & COLUMNS



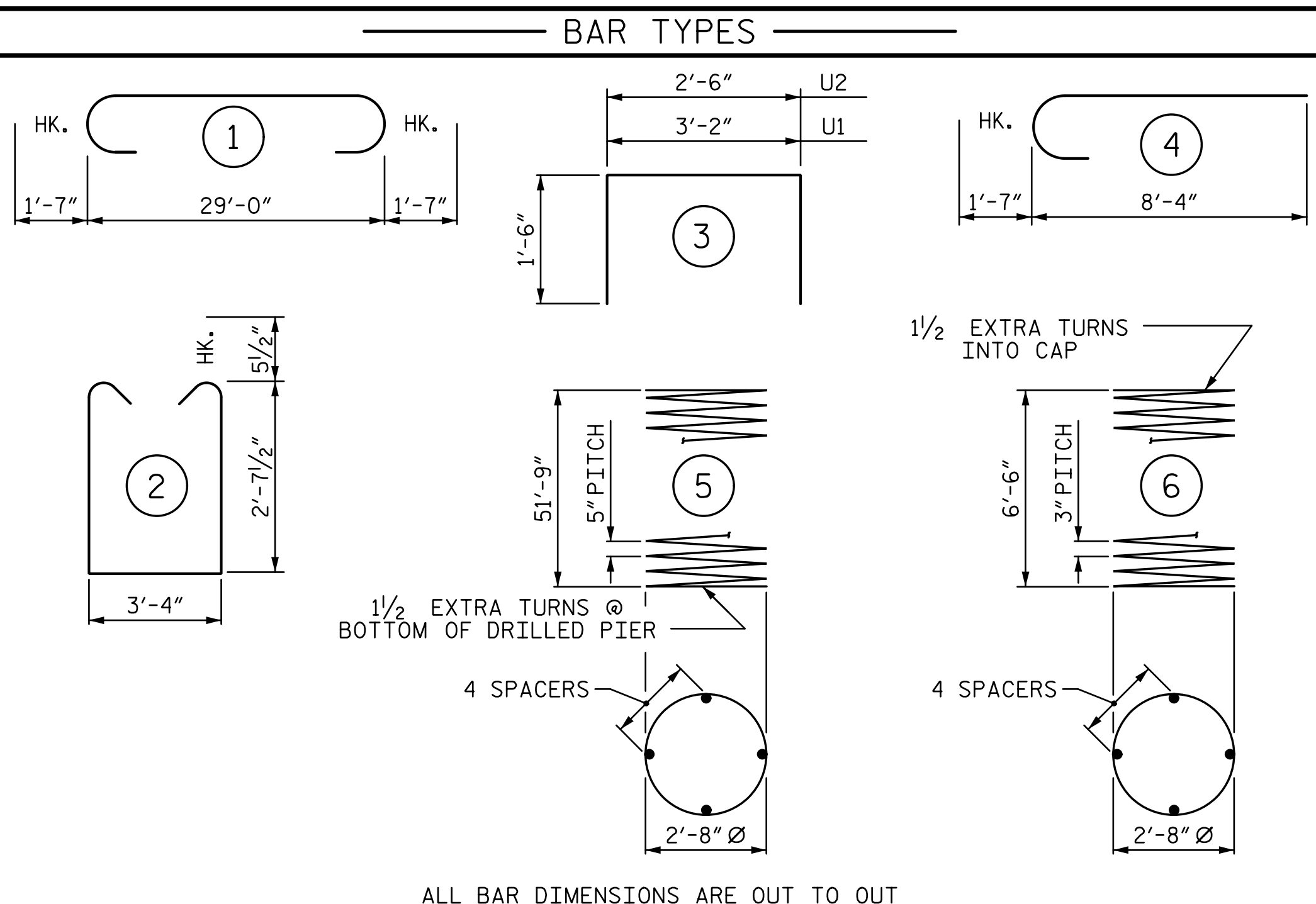
END ELEVATION



CONSTRUCTION JOINT DETAIL



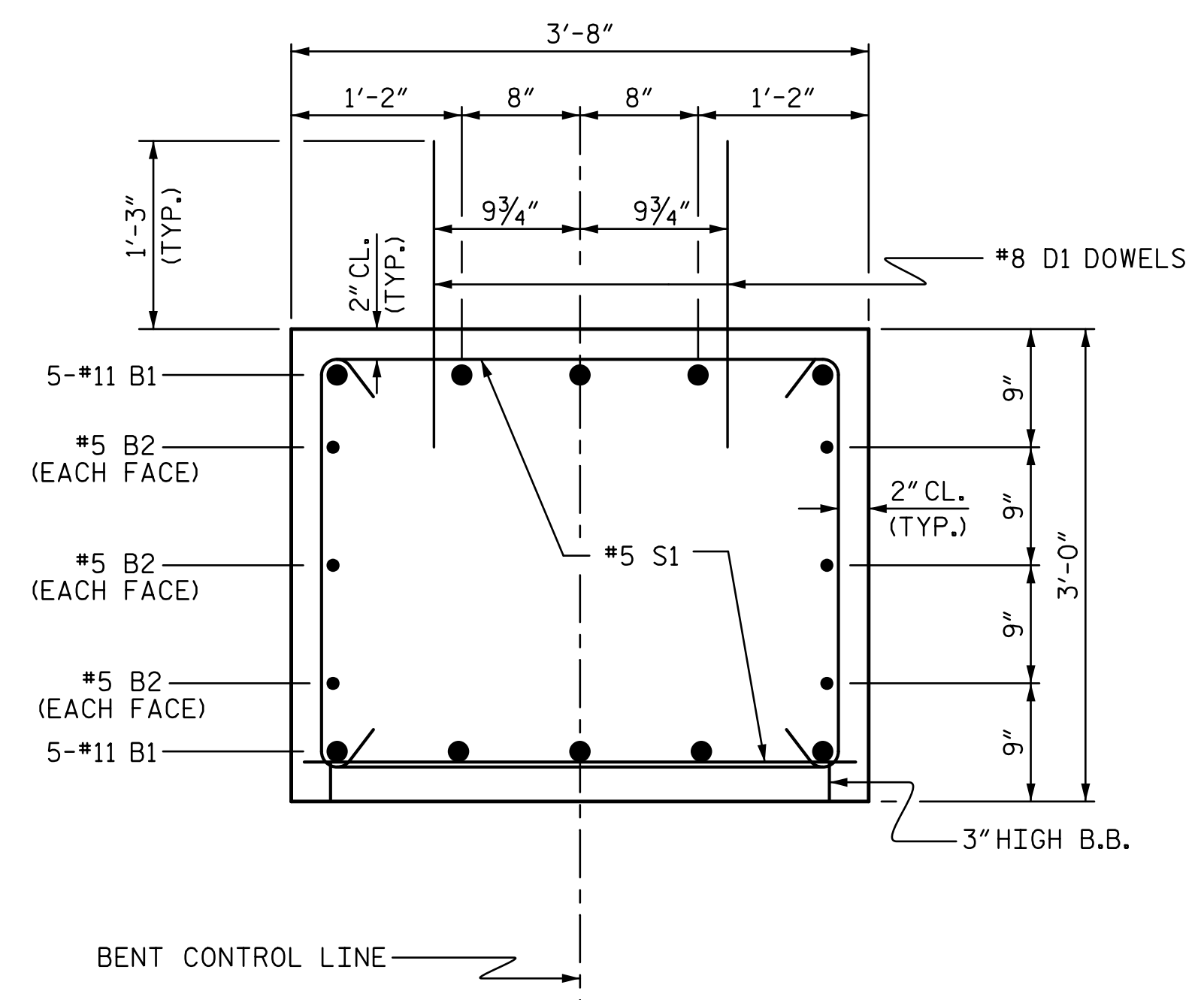
END OF CAP VIEW
(TYPICAL BOTH ENDS)



ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL

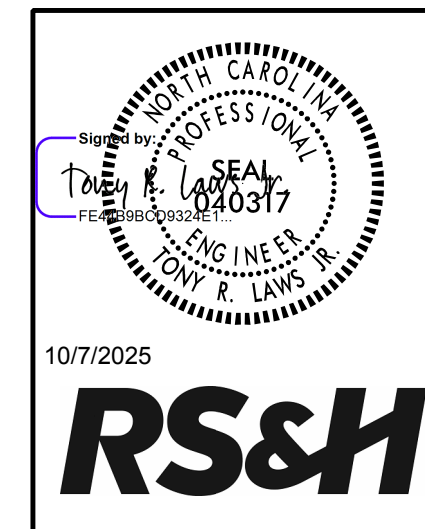
BENT 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	10	#11	1	32'-2"	1709
B2	6	#5	STR	29'-2"	183
D1	36	#8	STR	2'-3"	216
M1	30	#11	STR	59'-10"	9537
S1	46	#5	2	9'-6"	456
U1	6	#4	3	6'-2"	25
U2	8	#4	3	5'-6"	29
V1	30	#11	4	9'-11"	1581
REINFORCING STEEL					13,736 LBS.
SP-1	3	*	5	1036'-3"	3242
SP-2	3	**	6	231'-0"	463
SPIRAL REINFORCING STEEL					3,705 LBS.
* THE SP-1 SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR #5 PLAIN OR DEFORMED BAR					
** THE SP-2 SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR					
DRILLED PIER CONCRETE					
POUR #1 DRILLED PIERS					55.9 C.Y.
CLASS A CONCRETE BREAKDOWN					
POUR #2 COLUMNS					4.9 C.Y.
POUR #3 CAP					12.0 C.Y.
TOTAL CLASS A CONCRETE					16.9 C.Y.



SECTION THRU CAP

PROJECT NO. DF18314.2075090
POLK COUNTY
 STATION: 12+15.00 -L-

SHEET 2 OF 2



10/7/2025
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 North Carolina License No. 50753-F-0403-1-C-24

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUBSTRUCTURE					
BENT 1 DETAILS					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO.				
S-19				
TOTAL SHEETS				
24				

DRAWN BY : J. SCACCA DATE : 02/2025
 CHECKED BY : T. R. LAWS DATE : 02/2025
 DESIGN ENGINEER OF RECORD: T. R. LAWS DATE : 10/2025

DOCUMENT NOT CONSIDERED
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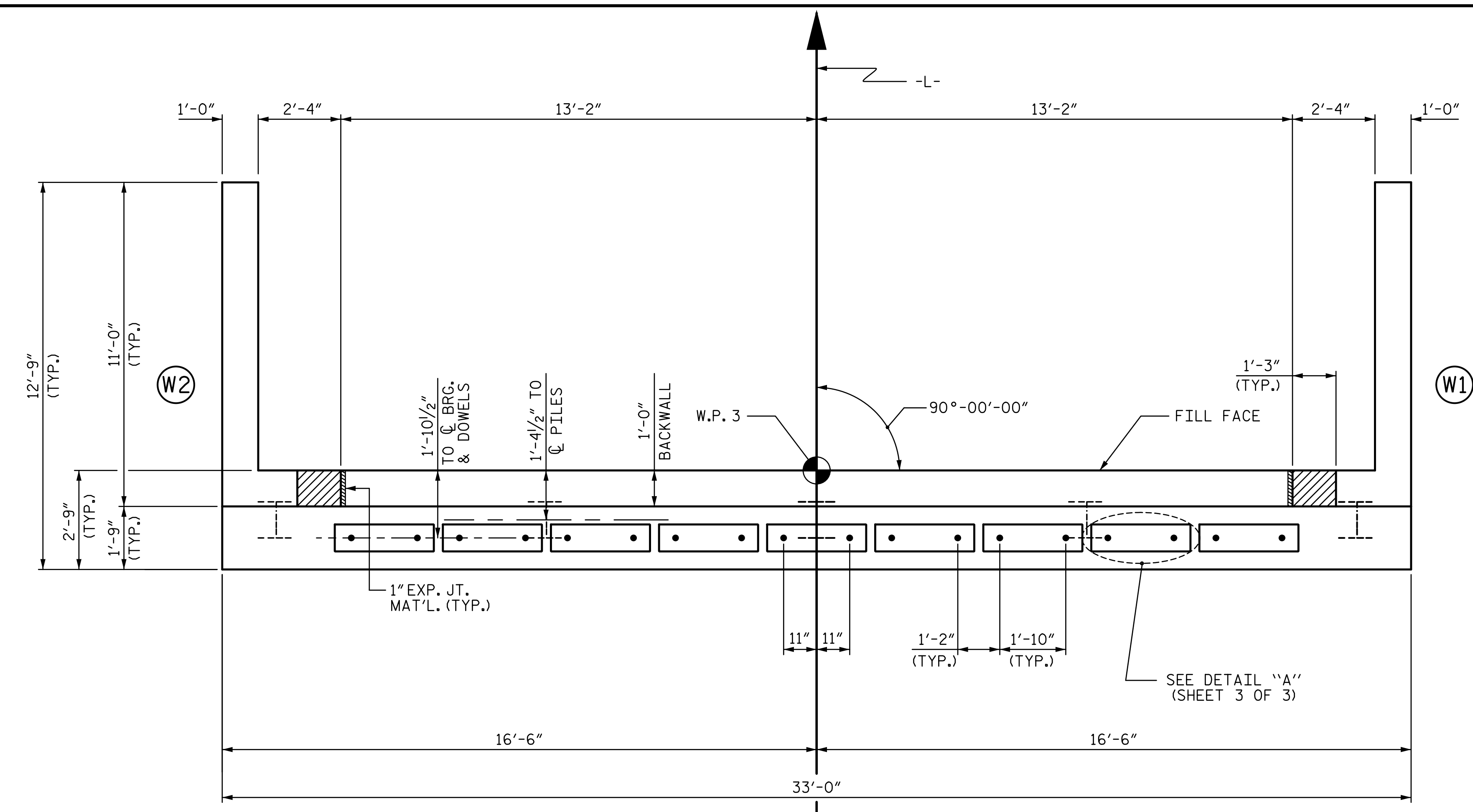
NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

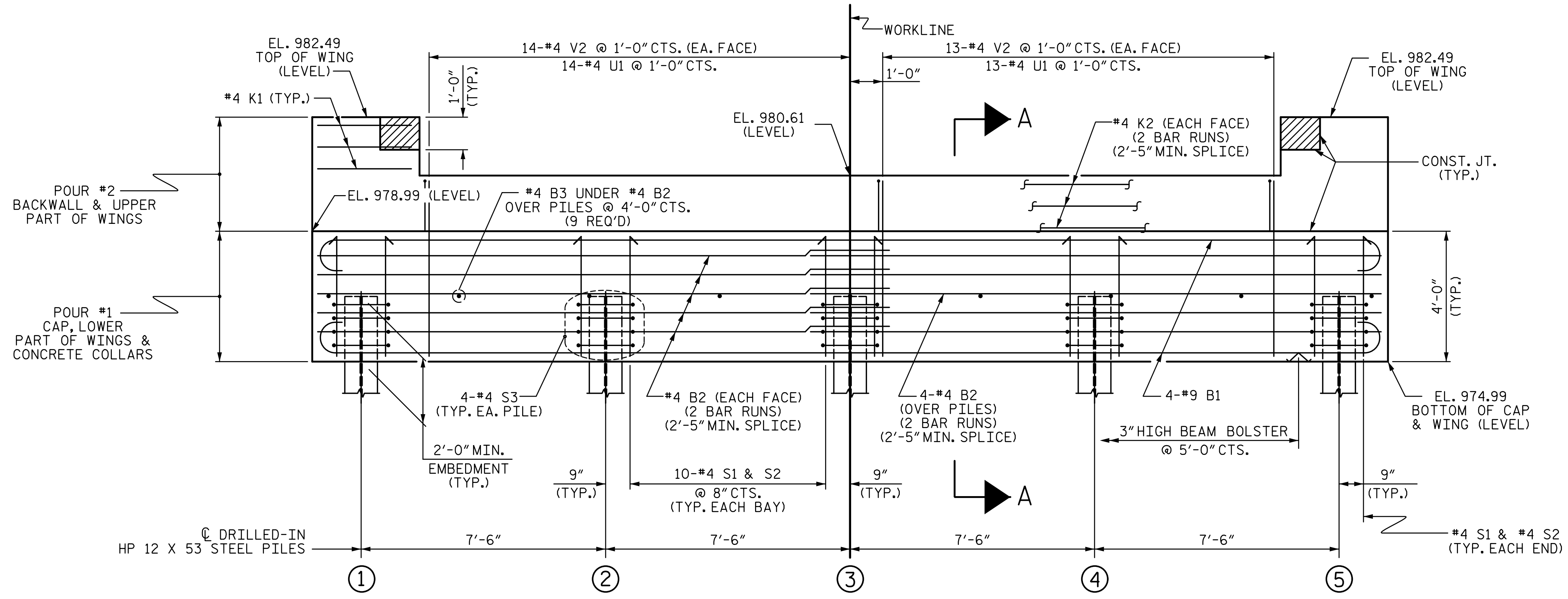
THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR WING DETAILS, SEE SHEET 2 OF 3.

FOR PILE SPLICE DETAILS, SEE SHEET 3 OF 3.



PLAN

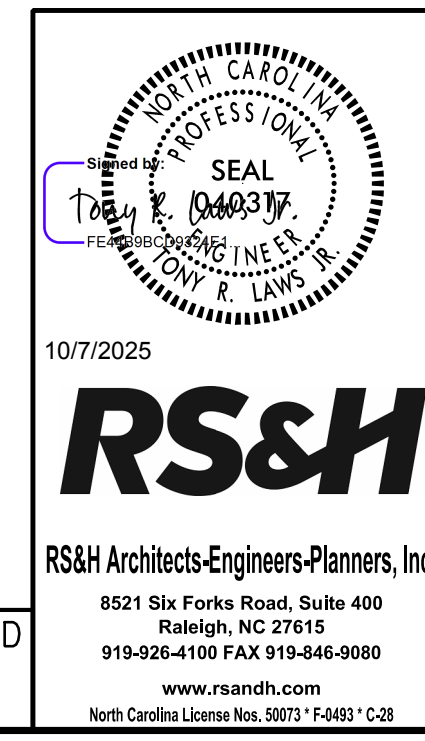


ELEVATION

WINGS NOT SHOWN FOR CLARITY.
FOR SECTION A-A, SEE SHEET 3 OF 3.
CONCRETE COLLARS FOR STEEL PILES NOT SHOWN IN PLAN AND ELEVATION VIEWS FOR CLARITY.
SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL", SHEET 3 OF 3.

PROJECT NO. DF18314.2075090
POLK COUNTY
STATION: 12+15.00 -L-

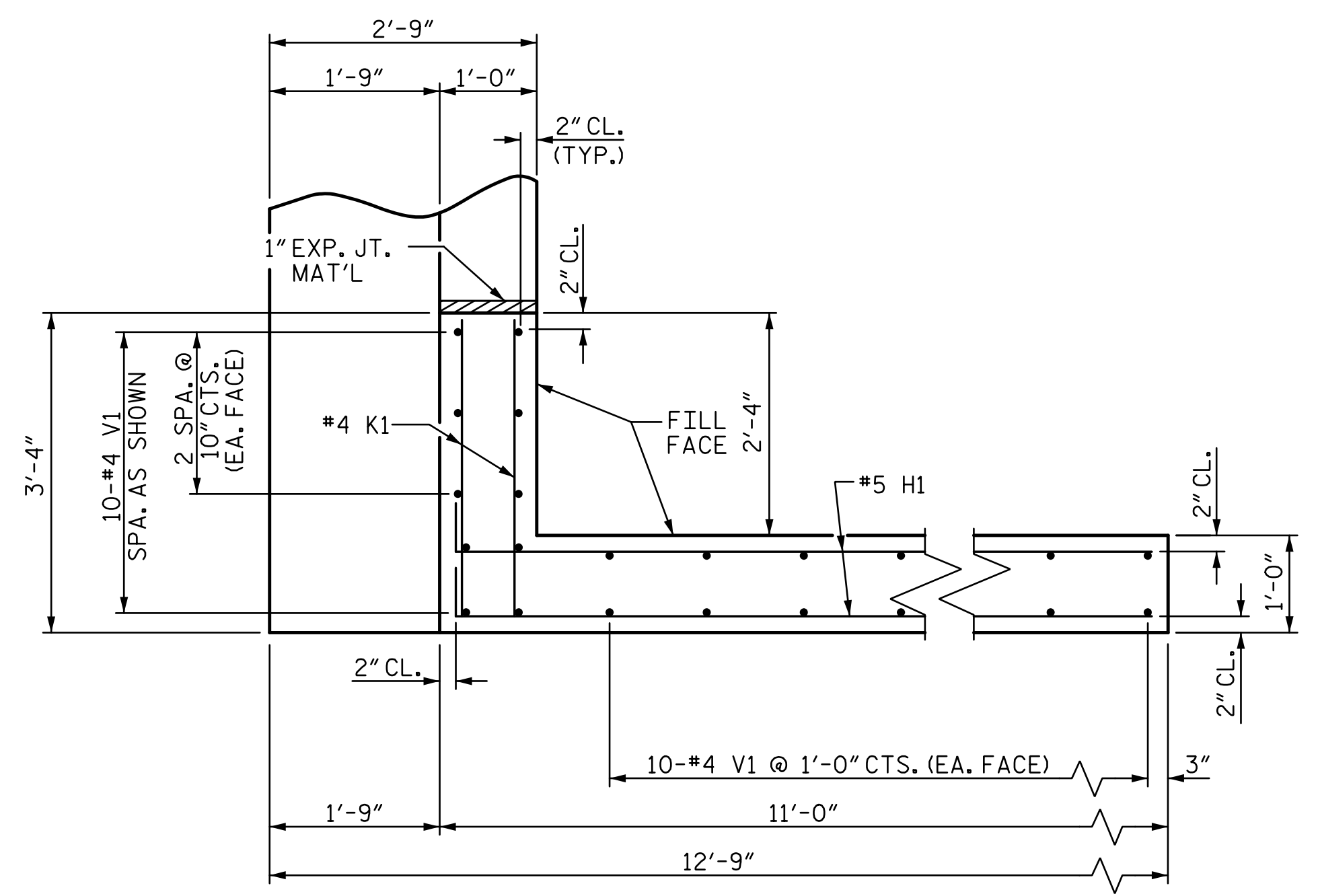
SHEET 1 OF 3



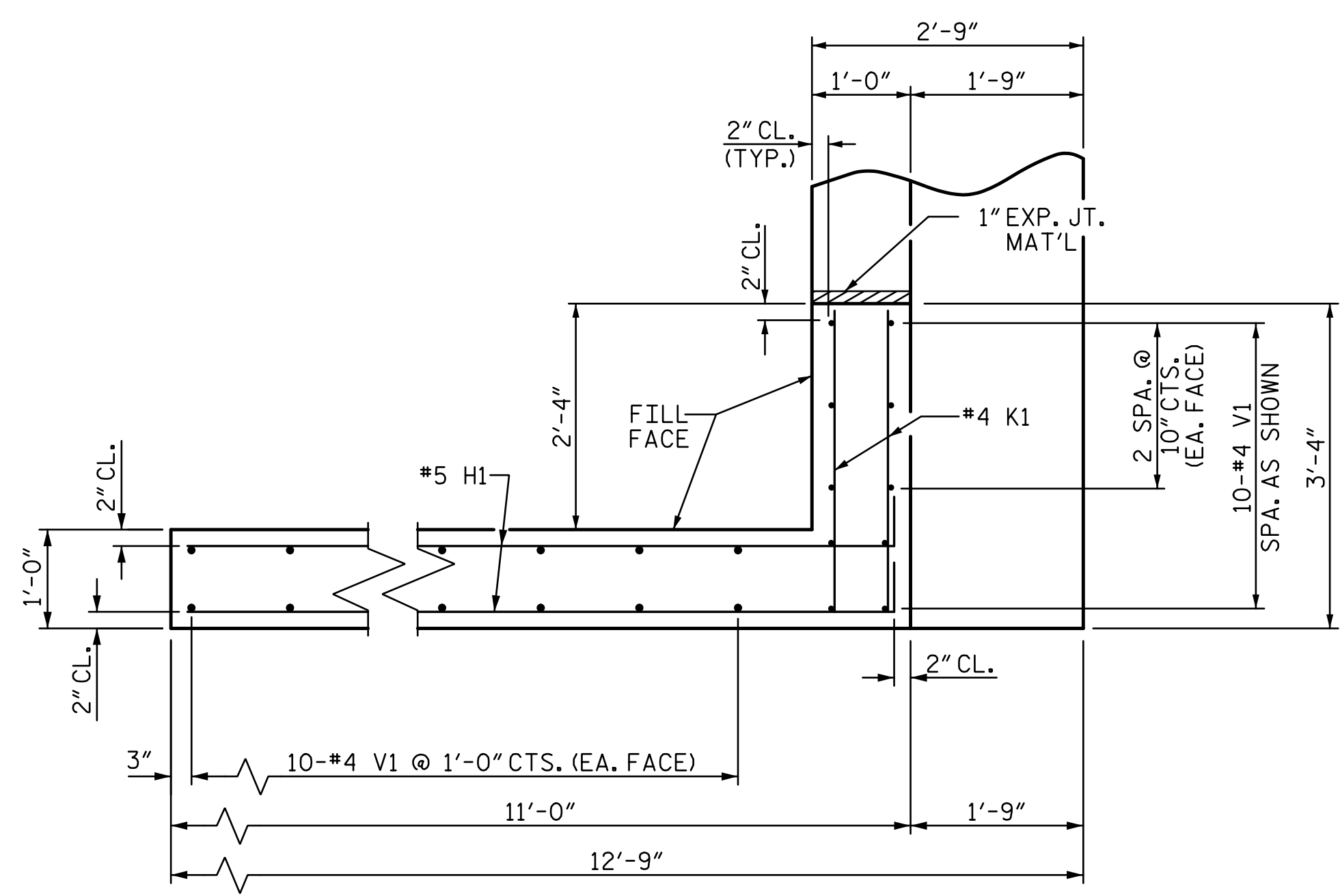
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REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO.					S-20
TOTAL SHEETS					24

DRAWN BY : J. SCACCA DATE : 05/2025
CHECKED BY : M. ACOSTA DATE : 07/2025
DESIGN ENGINEER OF RECORD: T. R. LAWS DATE : 10/2025

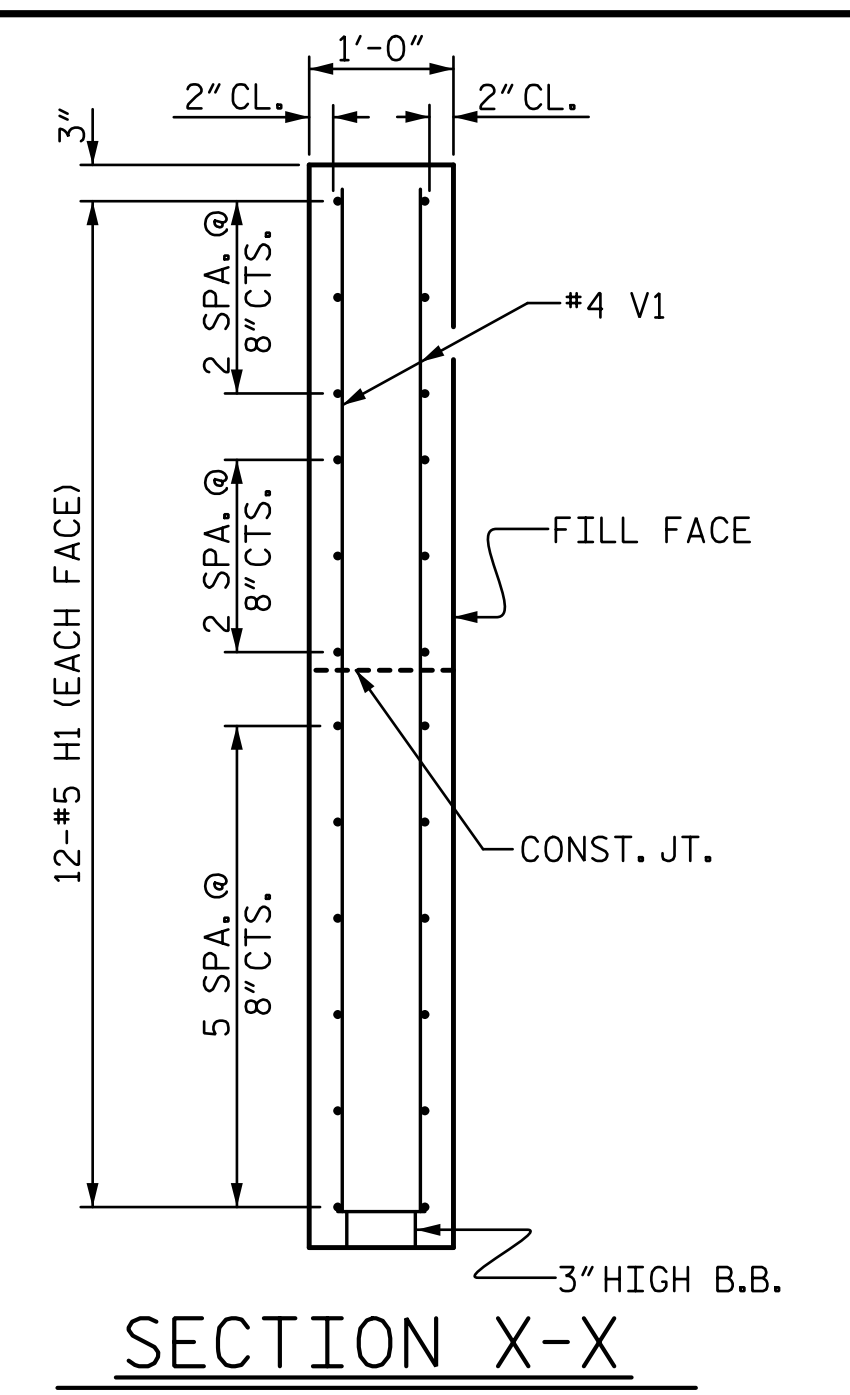
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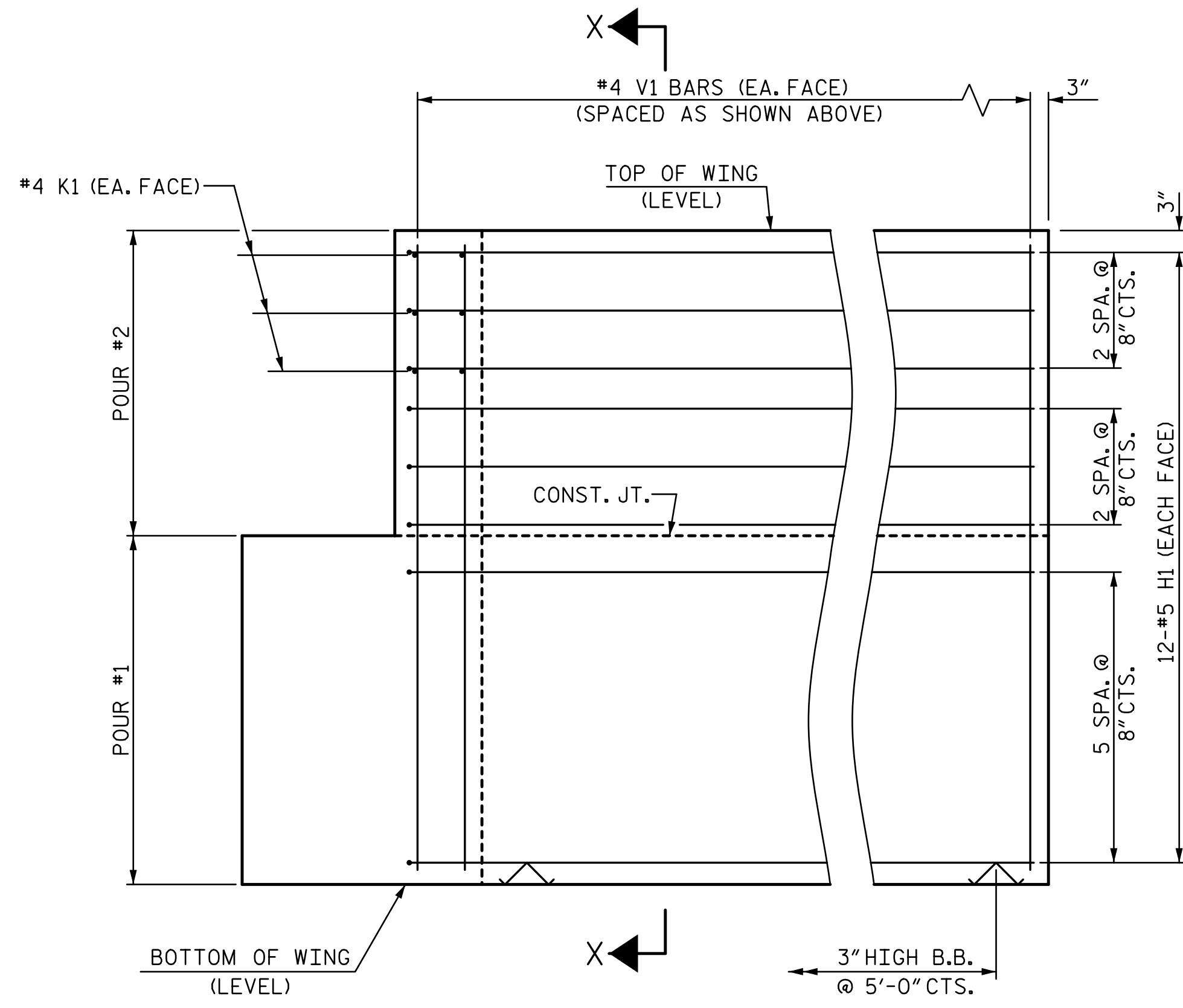
PLAN OF WING (W1)



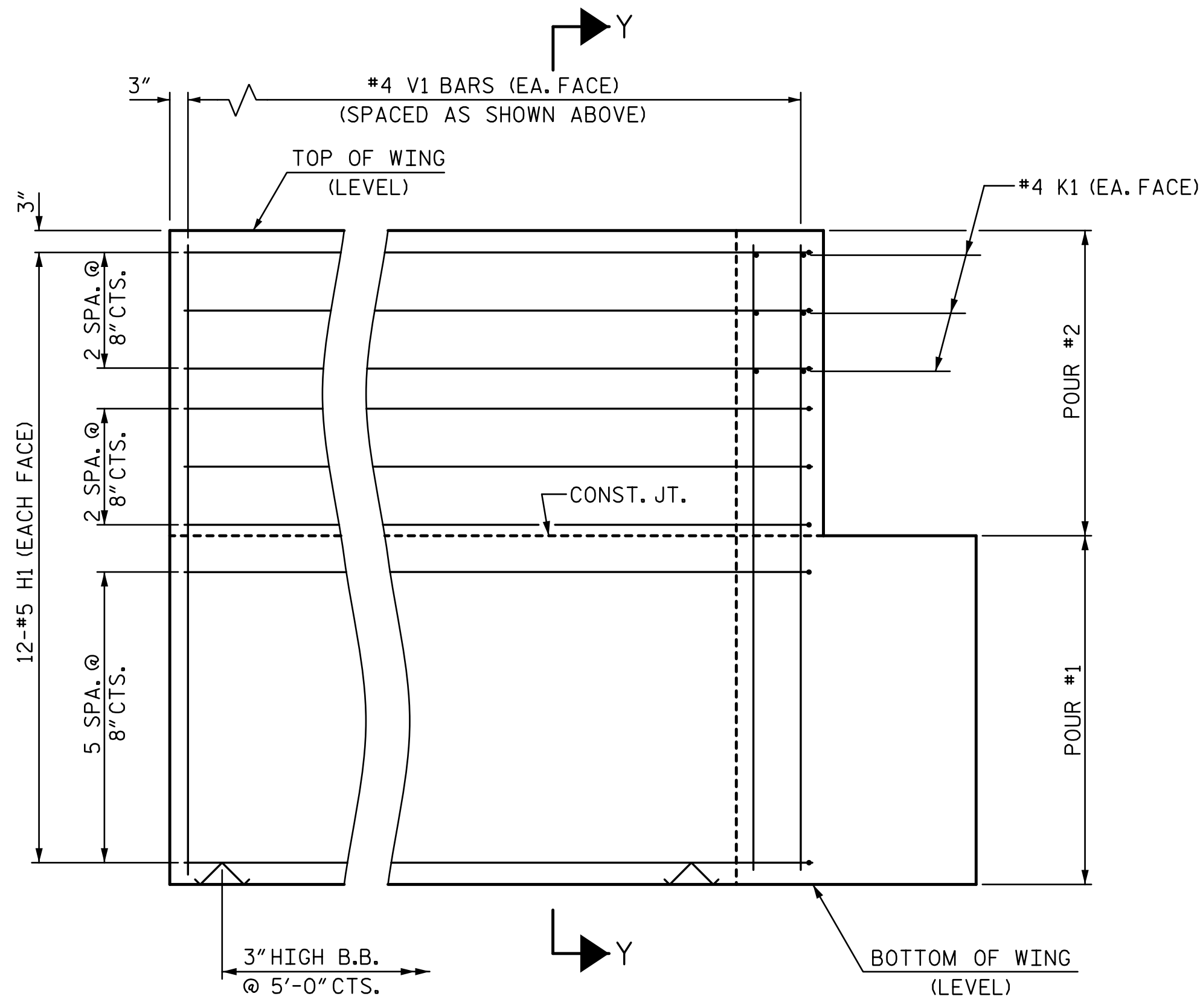
PLAN OF WING (W2)



SECTION X-X

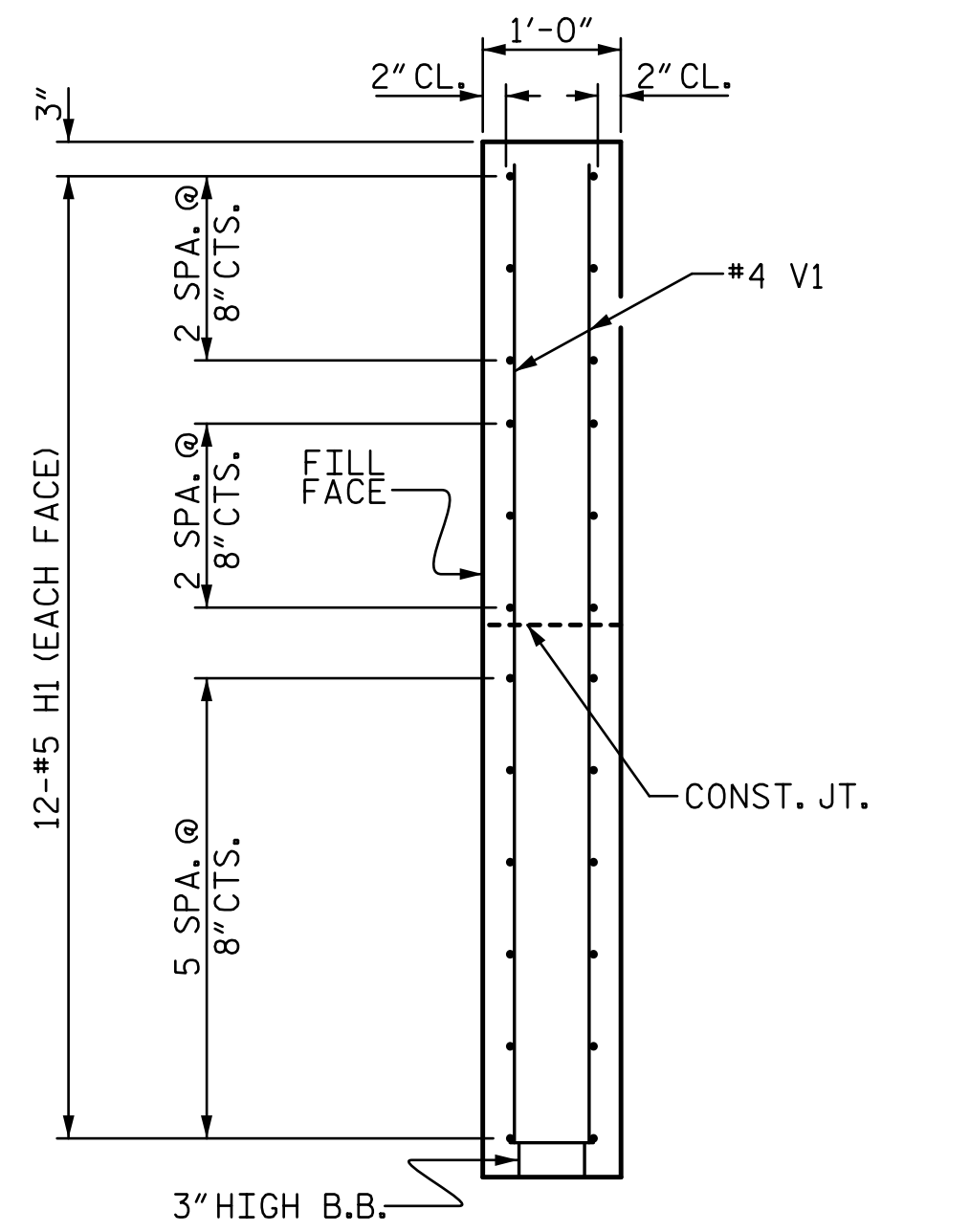


ELEVATION OF WING (W1)



ELEVATION OF WING (W2)

WING DETAILS



SECTION Y-Y

PROJECT NO. DF18314.2075090
POLK COUNTY
 STATION: 12+15.00 -L-

SHEET 2 OF 3

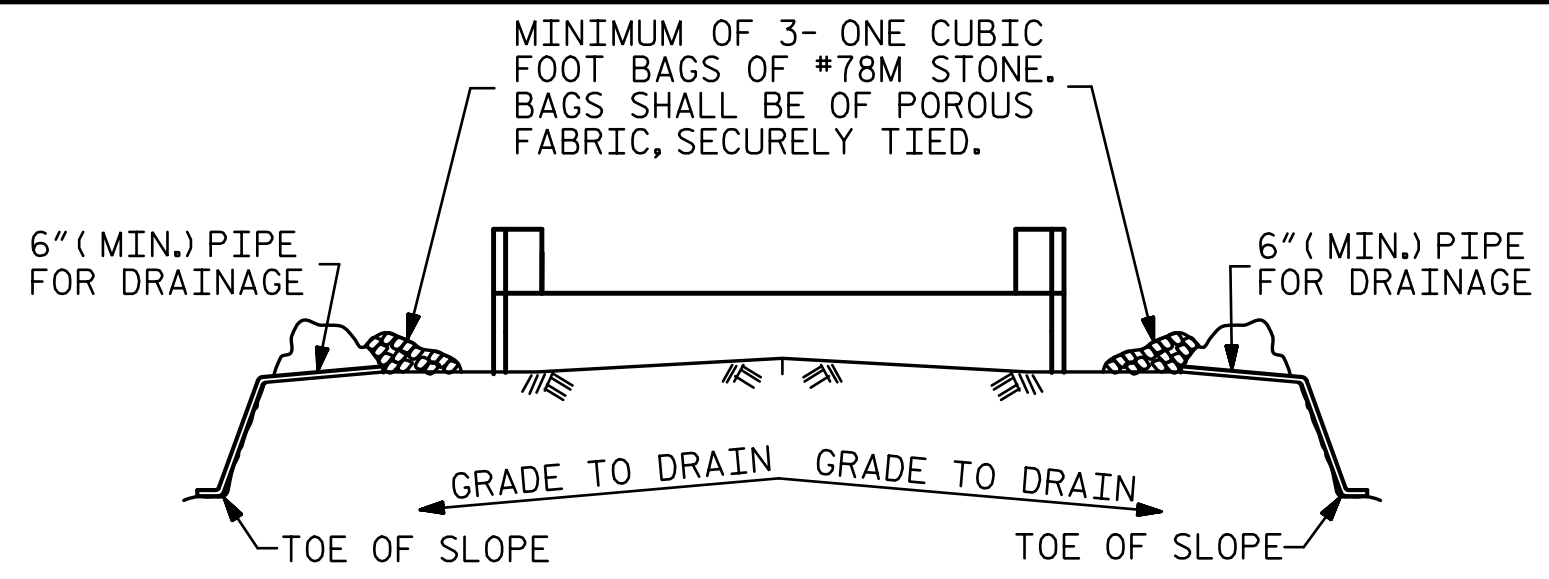


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 END BENT 2
 WING DETAILS

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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-21
1			3			TOTAL SHEETS
2			4			24

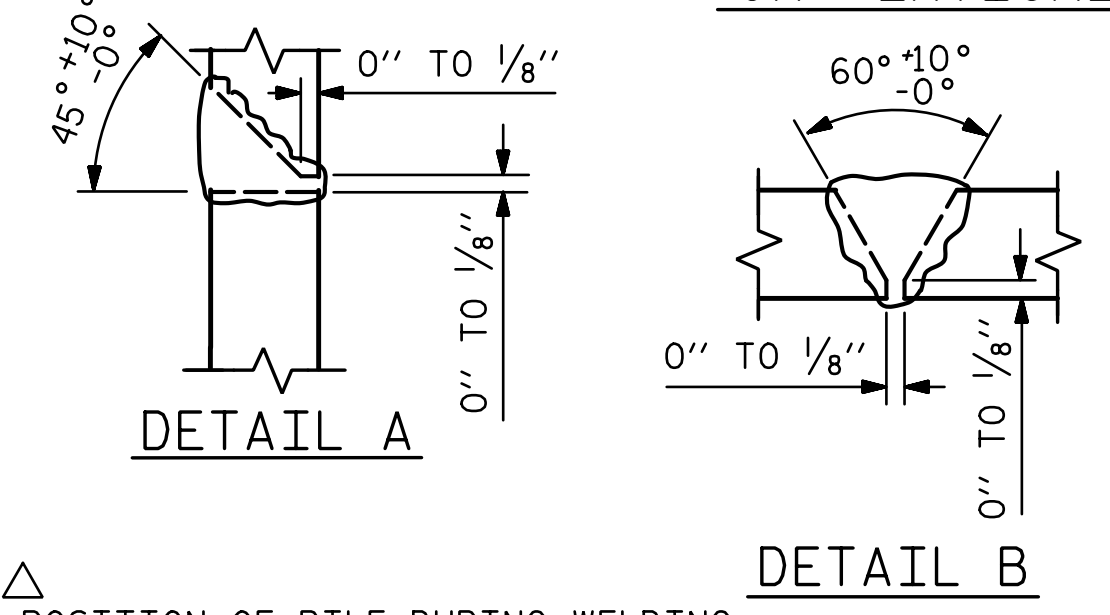
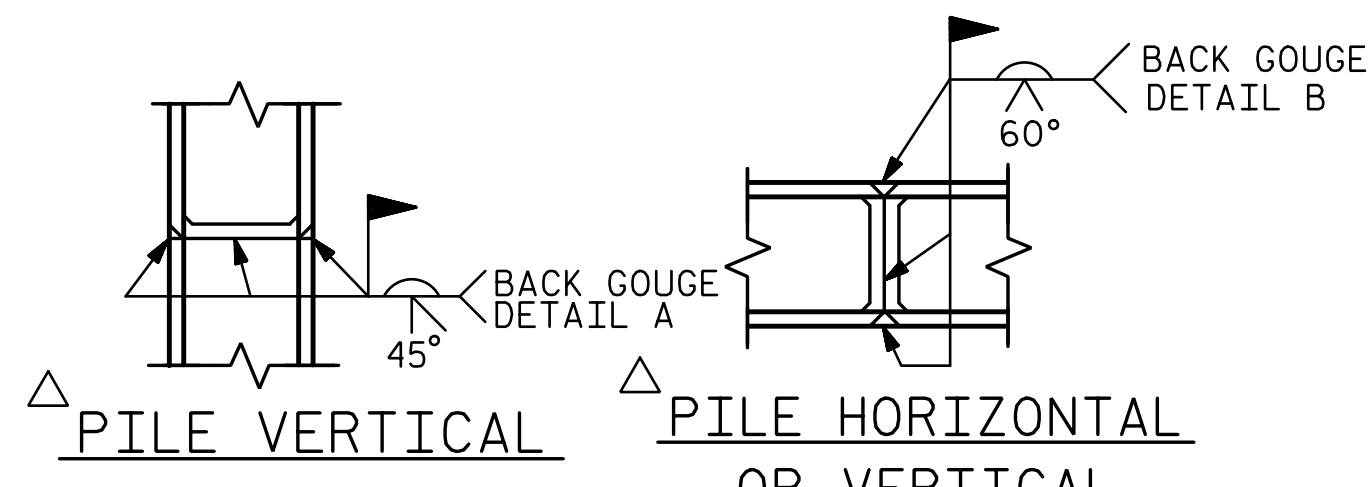


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

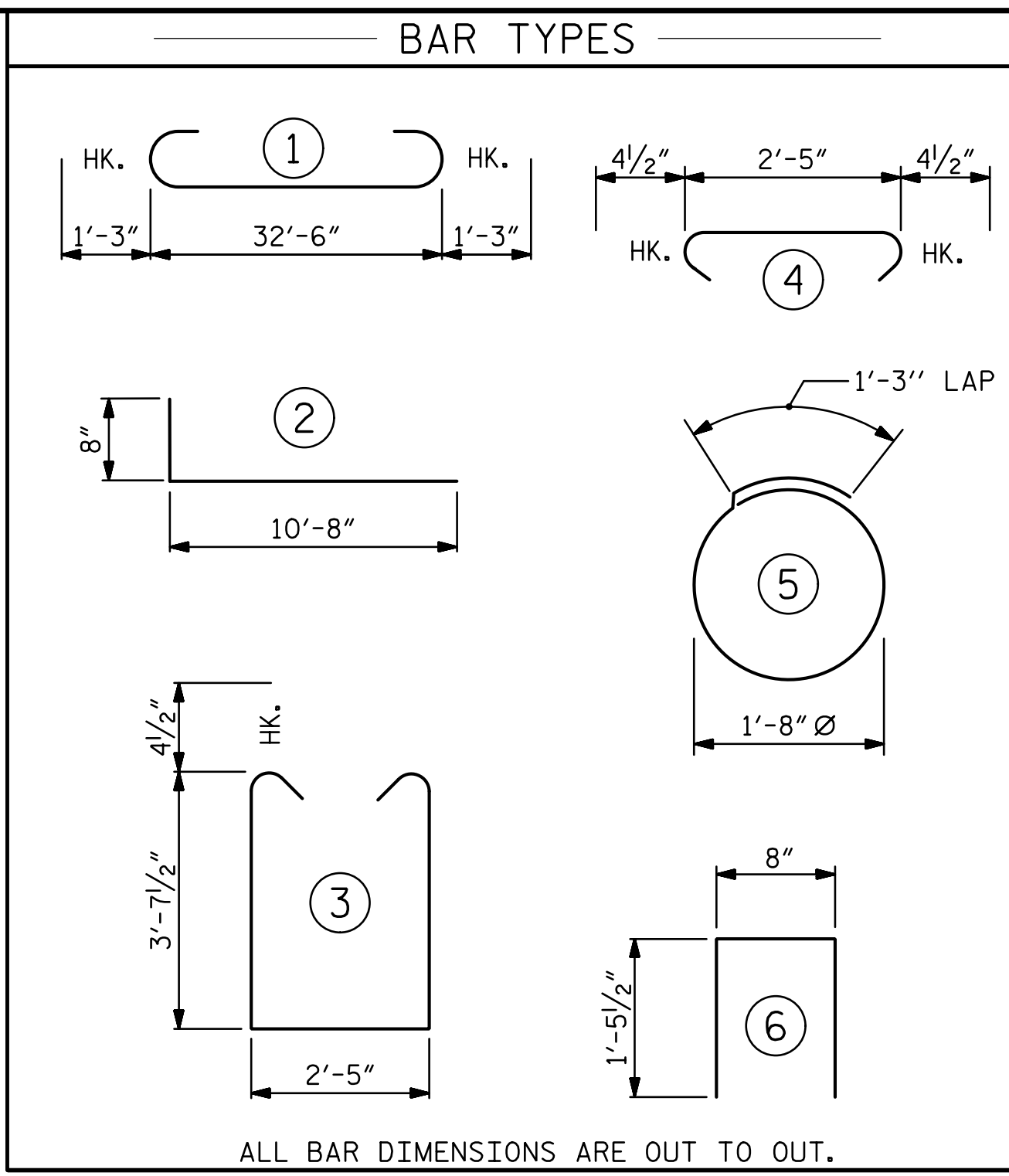
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

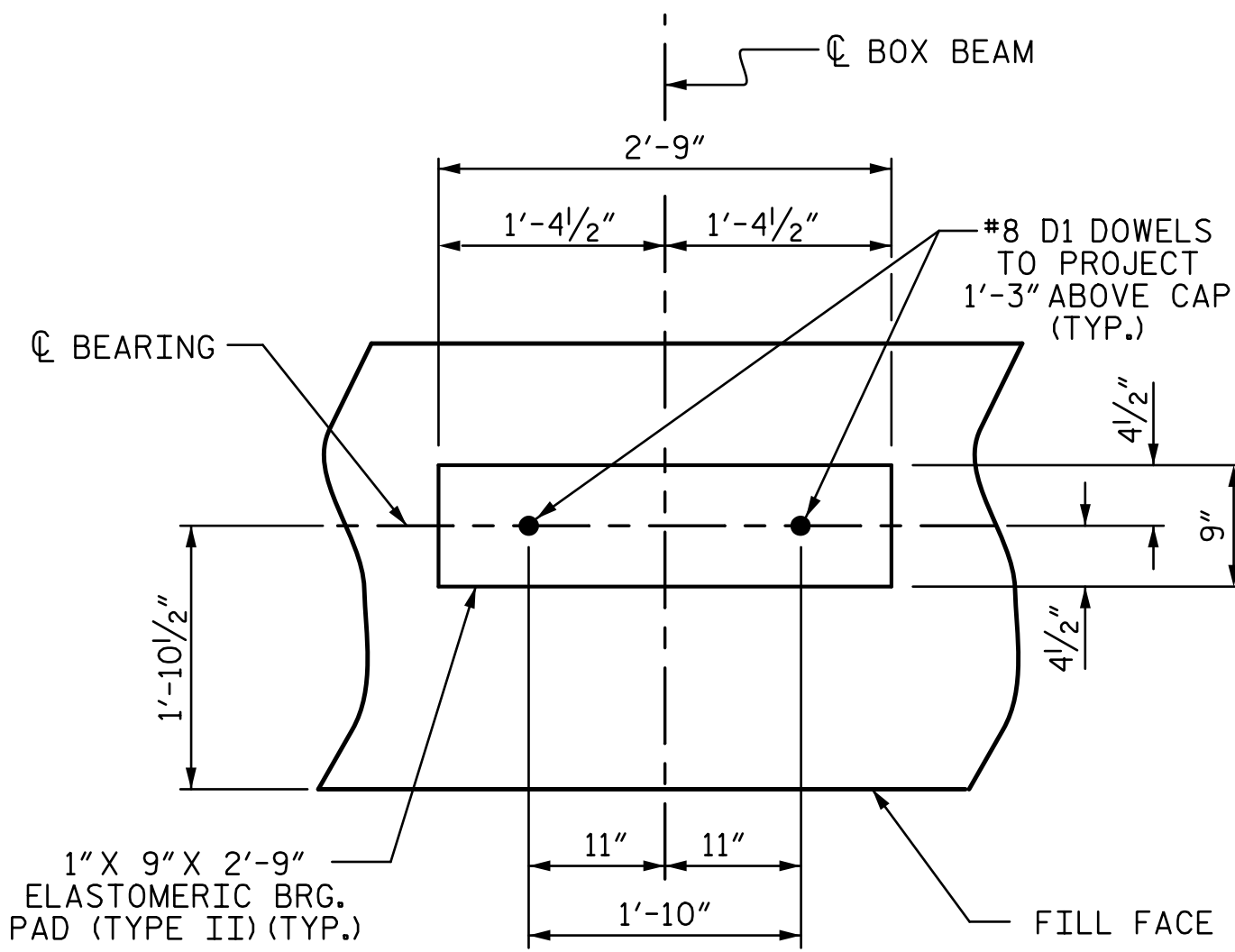
TEMPORARY DRAINAGE AT END BENT



PILE SPLICE DETAILS

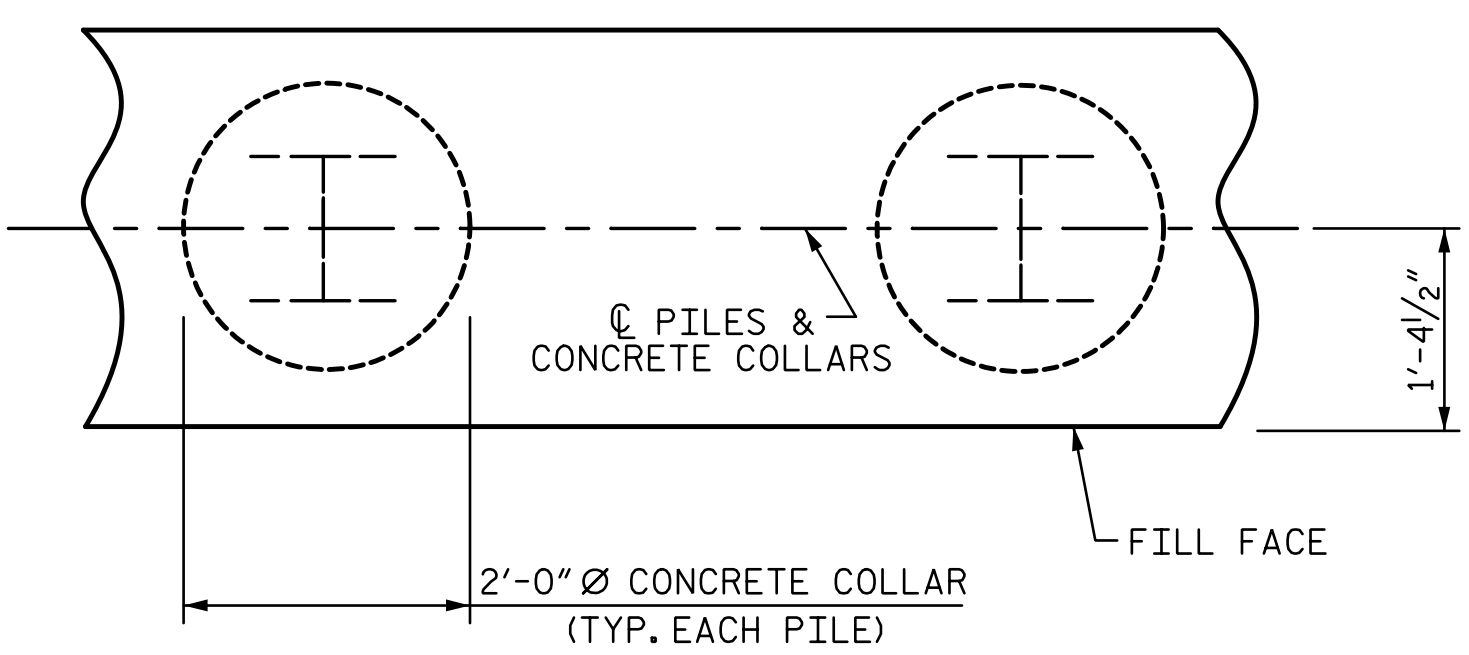


BILL OF MATERIAL					
END BENT 2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	8	#9		35'-0"	952
B2	28	#4	STR	17'-7"	329
B3	9	#4	STR	2'-5"	15
D1	18	#8	STR	2'-3"	108
H1	48	#5		11'-4"	567
K1	12	#4	STR	2'-11"	23
K2	12	#4	STR	17'-7"	140
S1	42	#4		10'-5"	292
S2	42	#4		3'-2"	89
S3	20	#4		6'-6"	87
U1	27	#4		3'-7"	65
V1	60	#4	STR	7'-2"	287
V2	54	#4	STR	5'-3"	189
REINFORCING STEEL (FOR ONE END BENT)					3,143 LBS.
CLASS A CONCRETE BREAKDOWN (FOR ONE END BENT)					
POUR #1	CAP, LOWER PART OF WINGS & COLLARS				17.3 C.Y.
POUR #2	BACKWALL & UPPER PART OF WINGS				5.0 C.Y.
TOTAL CLASS A CONCRETE					22.3 C.Y.



DETAIL "A"

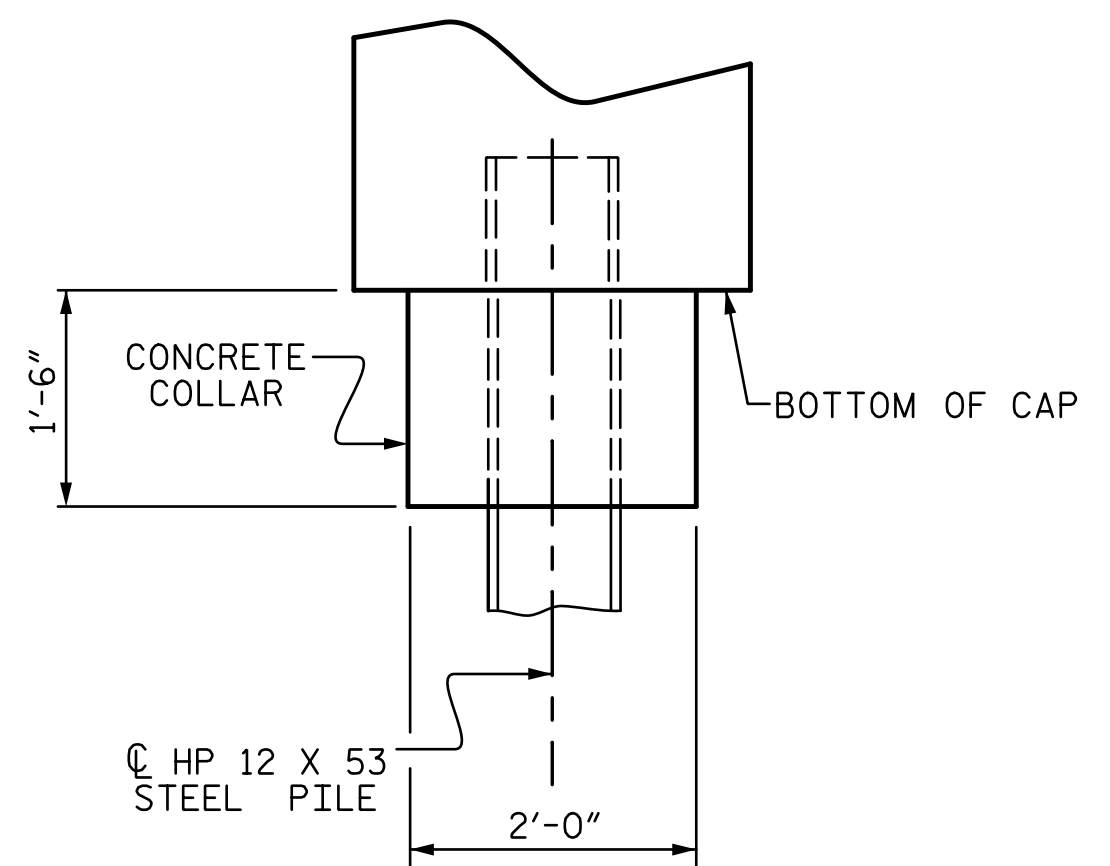
(END BENT 2 SHOWN)



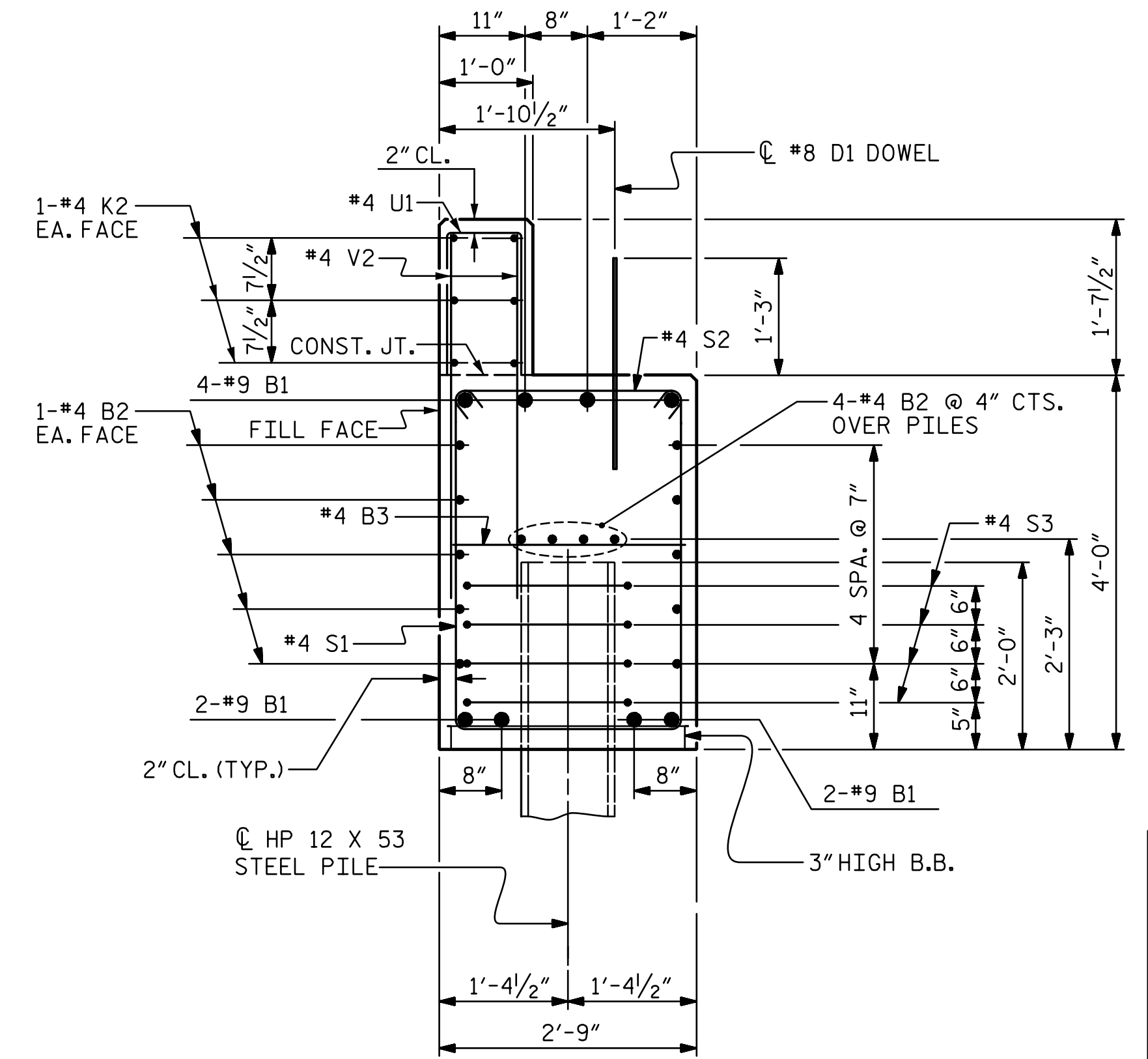
PLAN

CORROSION PROTECTION FOR STEEL PILES DETAIL

(END BENT 2 SHOWN)



ELEVATION



SECTION A-A

(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")

PROJECT NO. DF18314.2075090

POLK COUNTY

STATION: 12+15.00 -L-

SHEET 3 OF 3

10/7/2025

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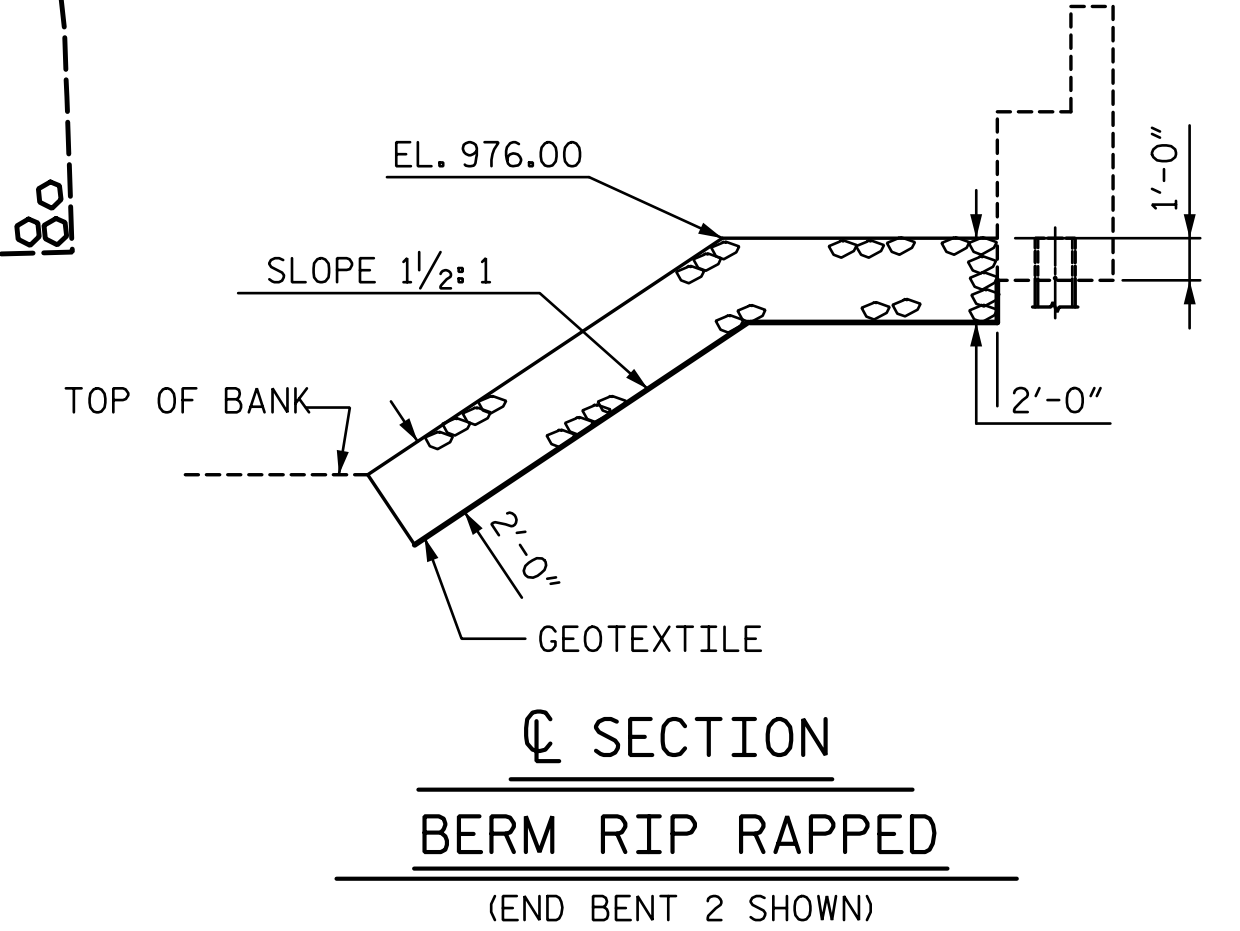
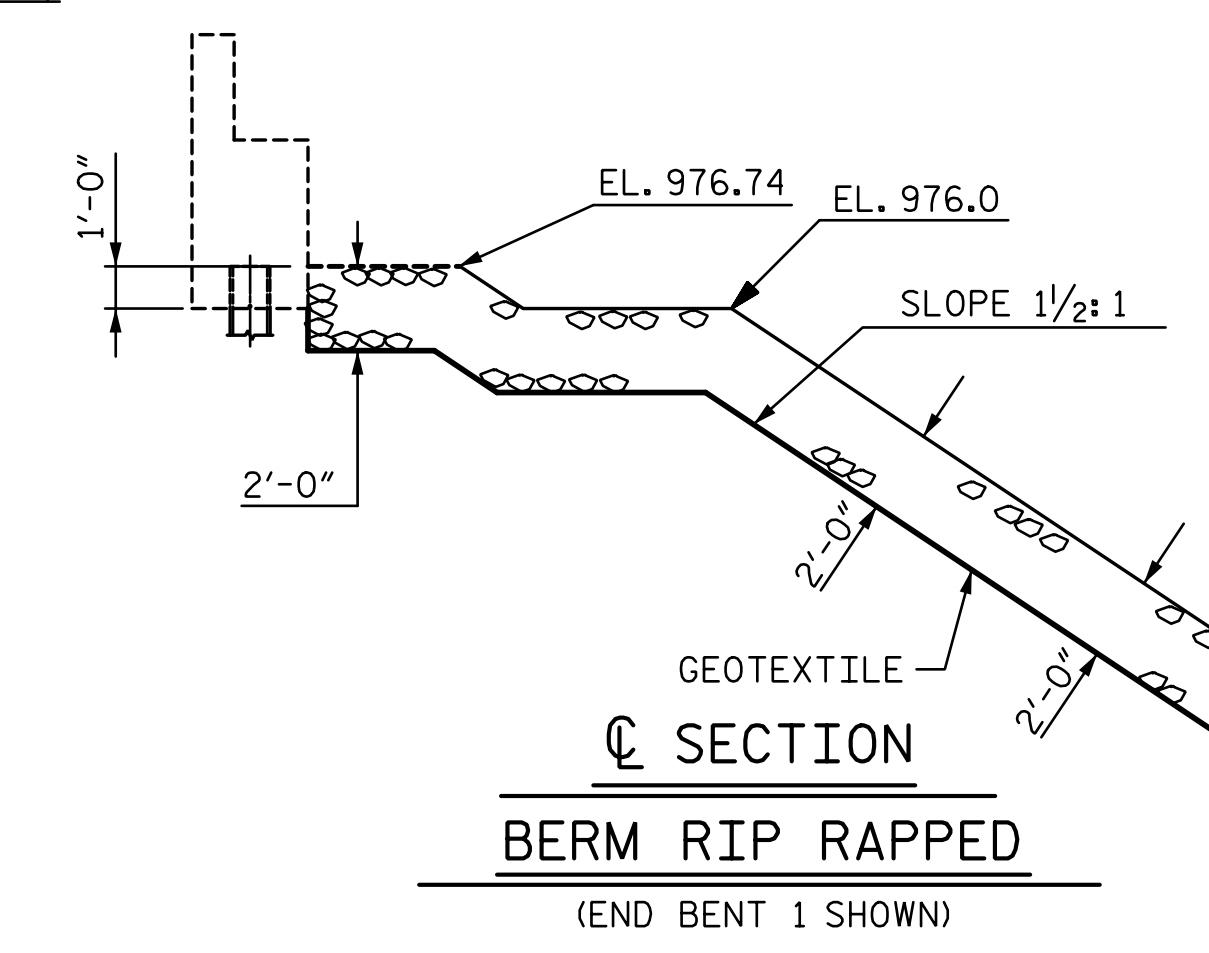
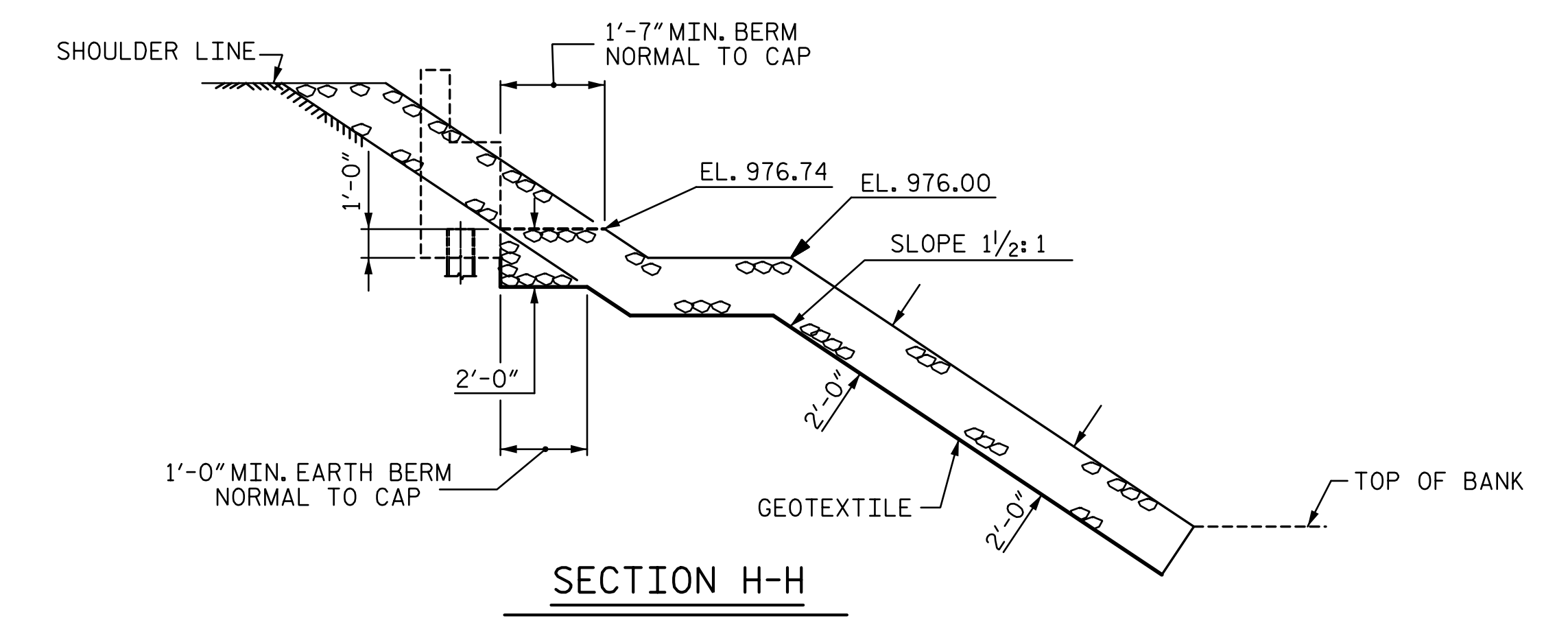
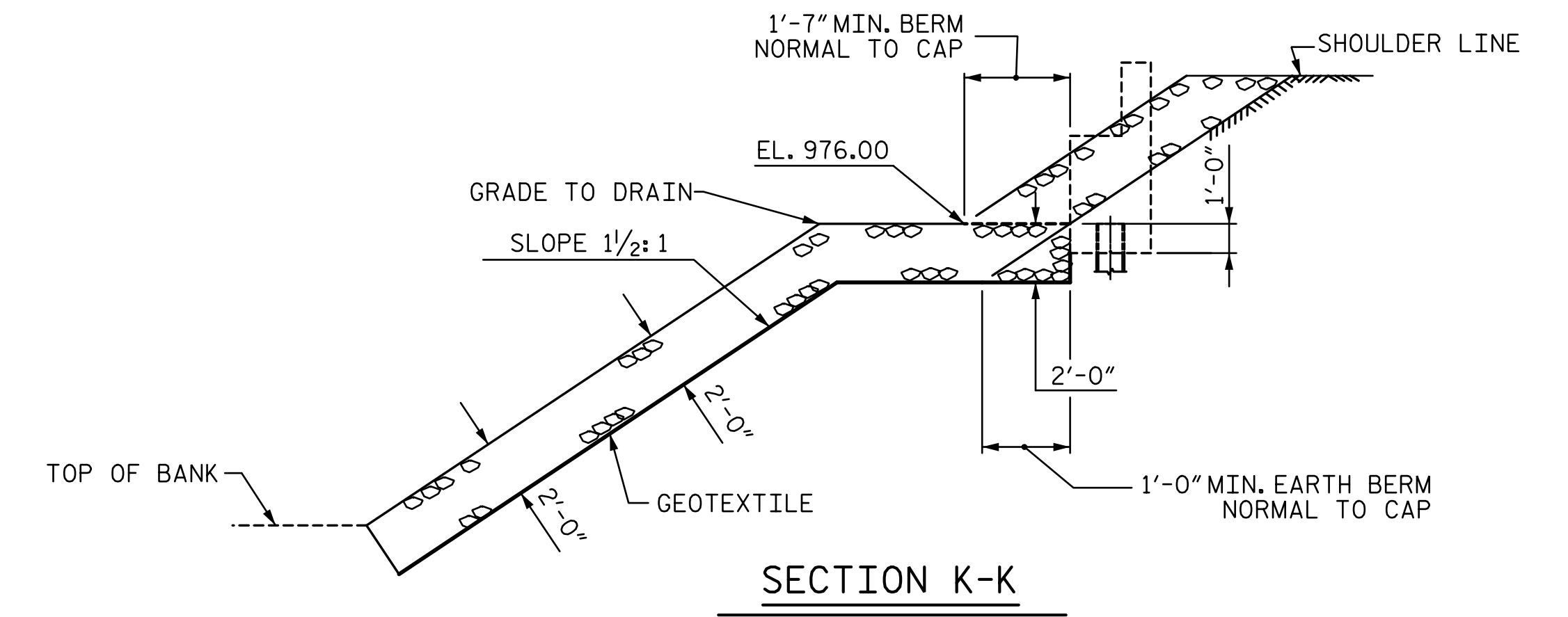
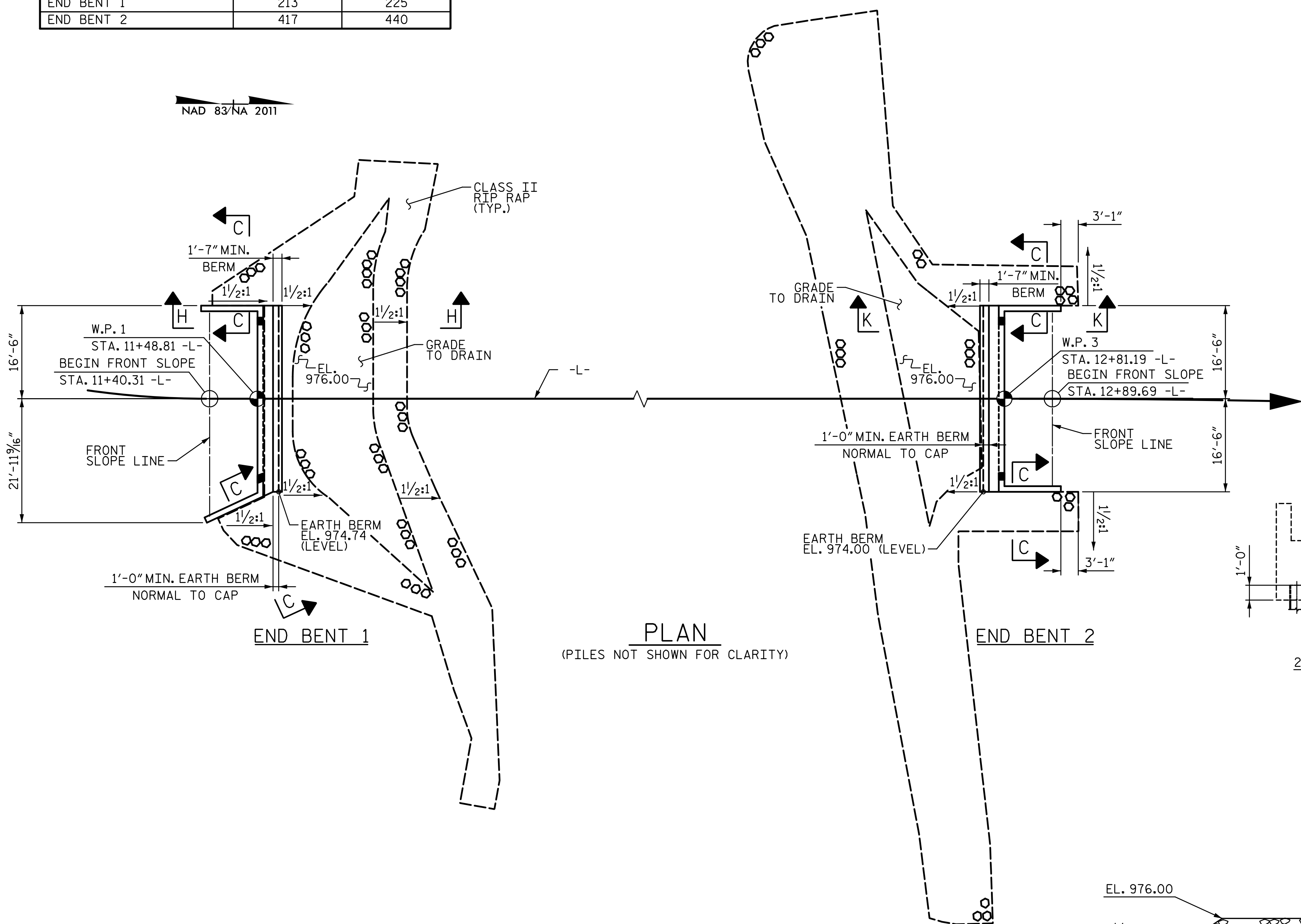
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUBSTRUCTURE					
END BENT 2 DETAILS					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					SHEET NO. S-22
					TOTAL SHEETS 24

DRAWN BY :	J. SCACCA	DATE :	05/2025
CHECKED BY :	M. ACOSTA	DATE :	07/2025
DESIGN ENGINEER OF RECORD:	T. R. LAWS	DATE :	10/2025

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ESTIMATED QUANTITIES		
BRIDGE @ STA. 12+15.00 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE
	TONS	SQUARE YARDS
END BENT 1	213	225
END BENT 2	417	440

NAD 83/NA 2011



PROJECT NO. DF18314.2075090
POLK COUNTY
 STATION: 12+15.00 -L-

10/7/2025

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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-23
1			3			TOTAL SHEETS
2			4			24

DRAWN BY : J. SCACCA DATE : 06/2025
 CHECKED BY : M. ACOSTA DATE : 06/2025
 DESIGN ENGINEER OF RECORD: T. R. LAWS DATE : 10/2025

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NOTES

FOR BRIDGE APPROACH FILL, INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND SELECT MATERIAL BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

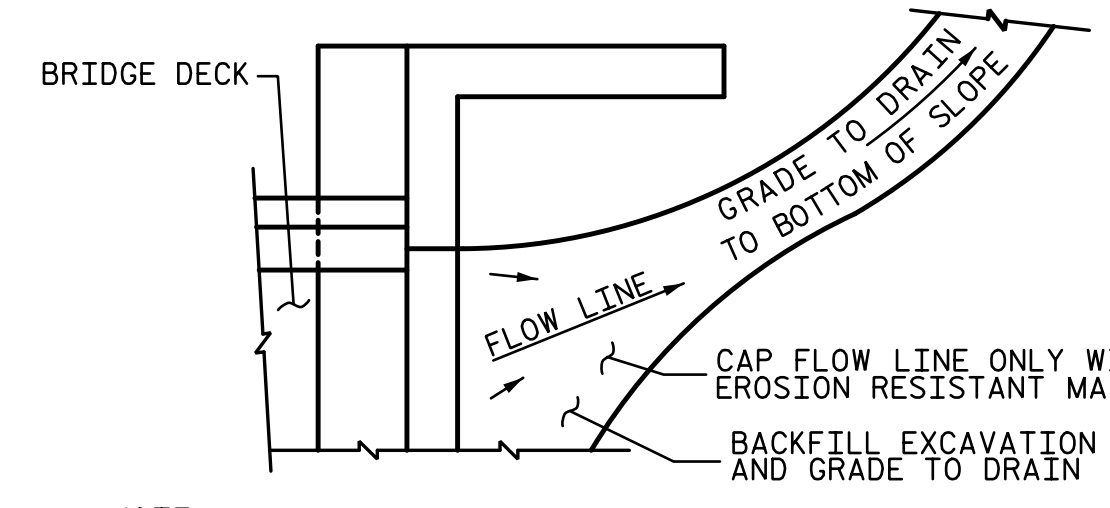
SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

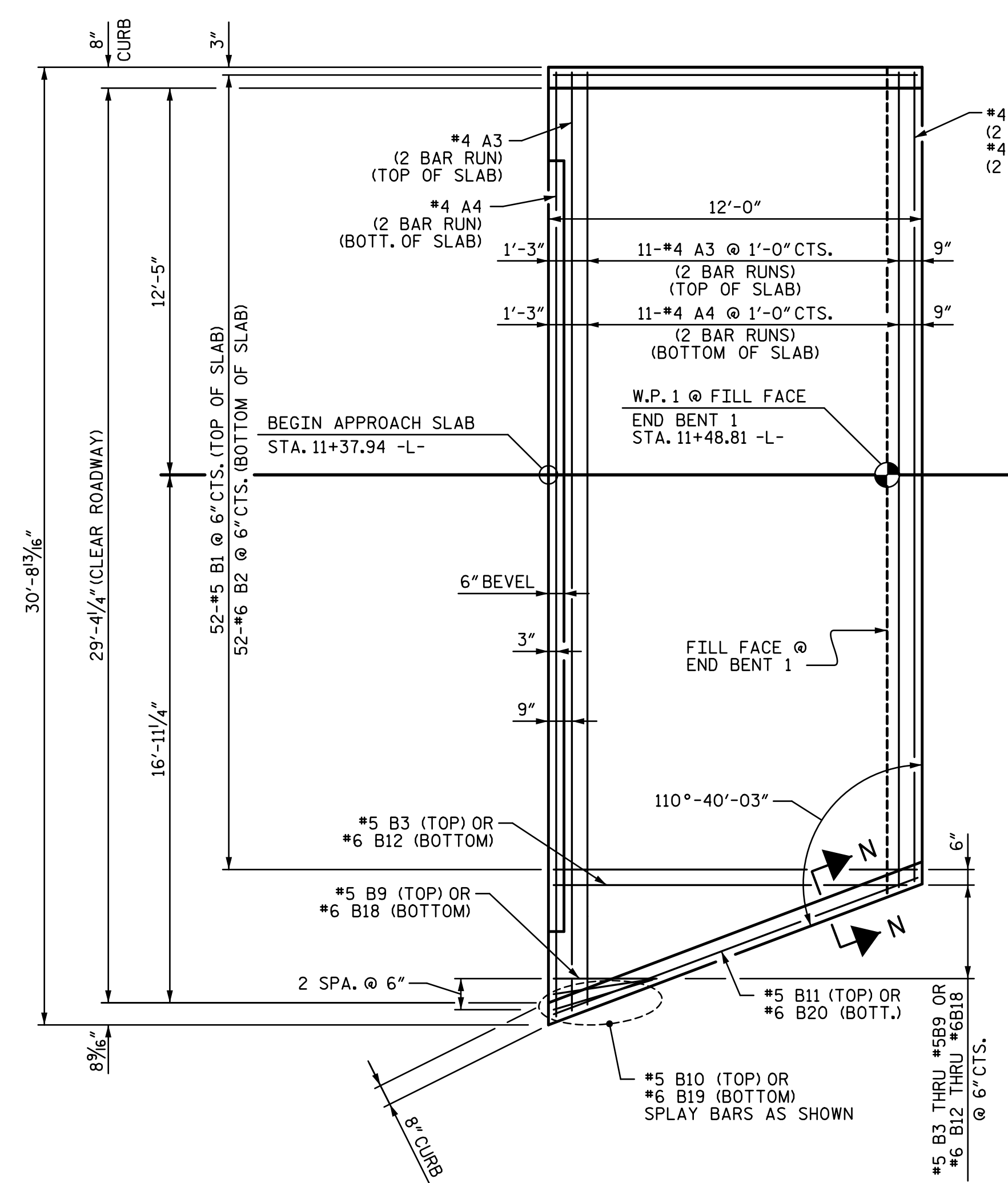
APPROACH SLAB GROOVING IS NOT REQUIRED.



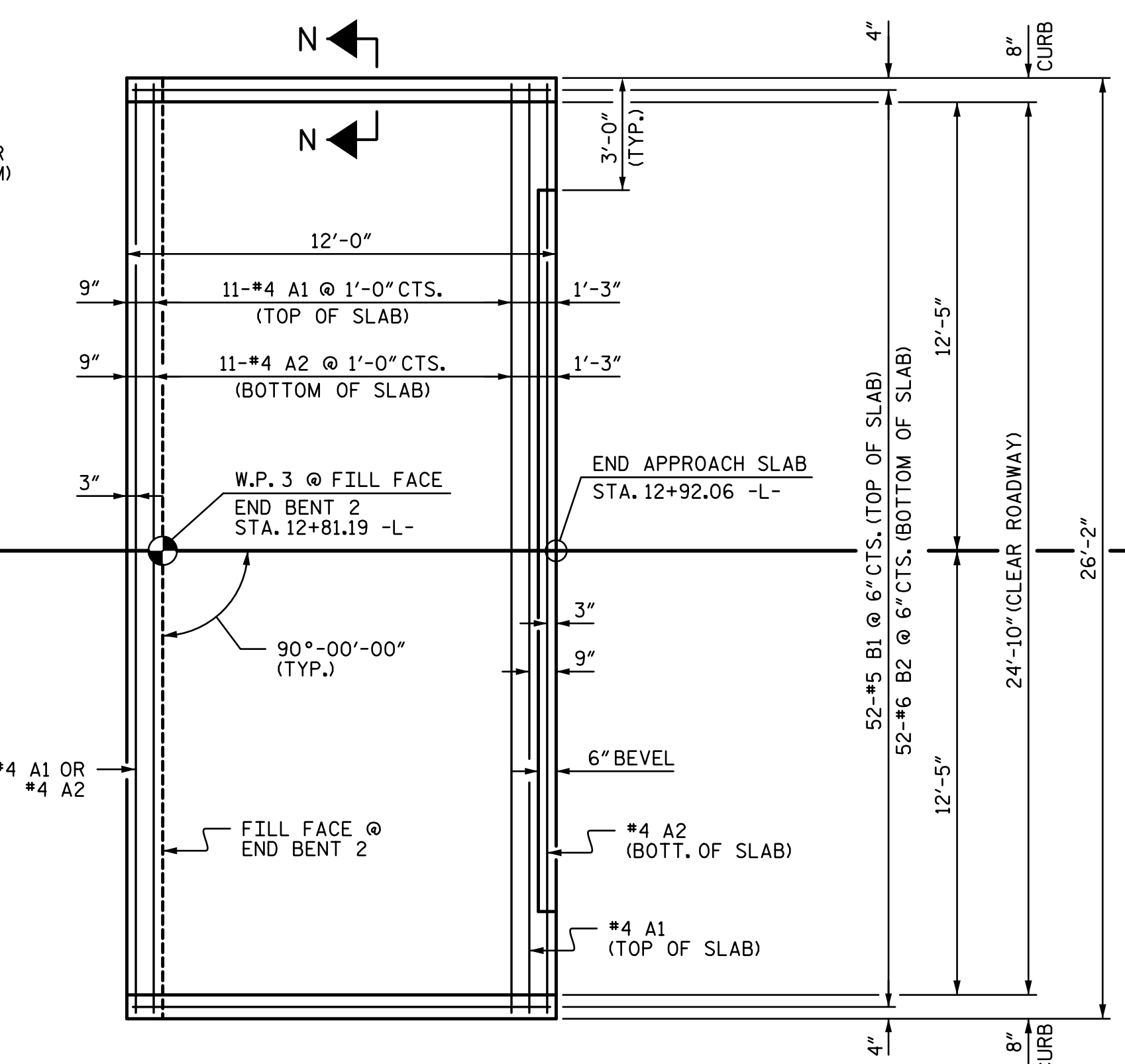
NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

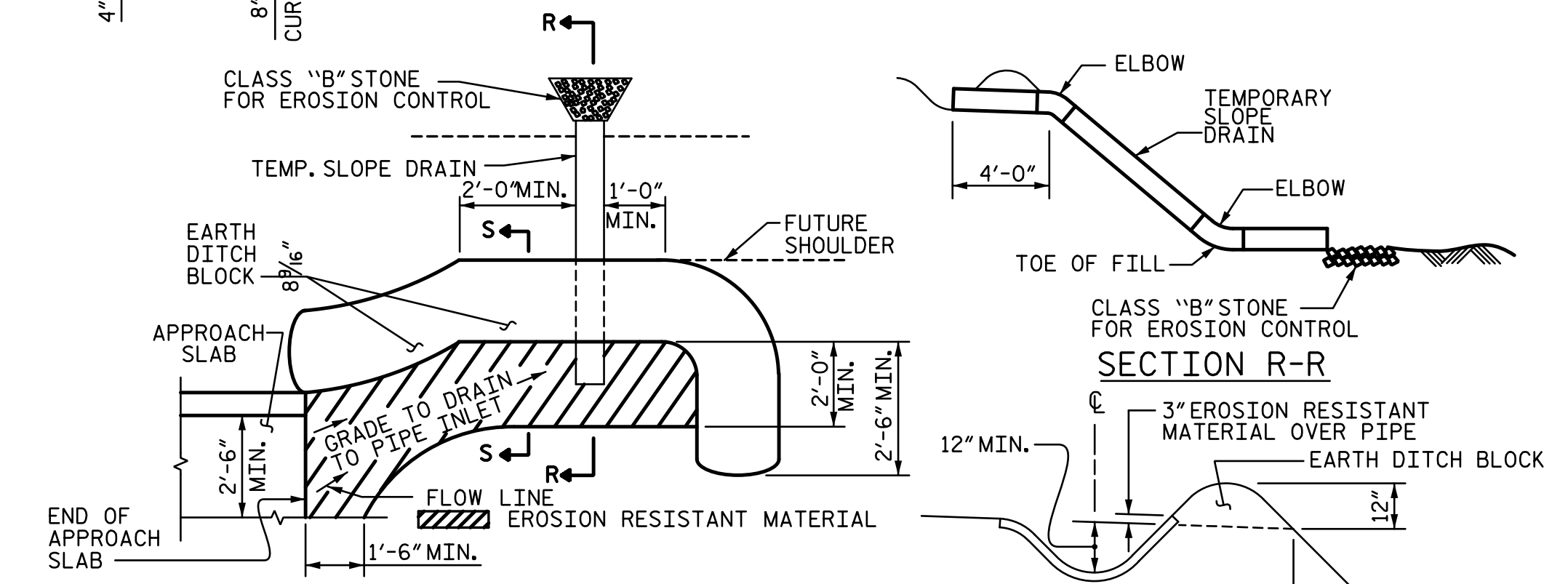
BILL OF MATERIAL					
APPROACH SLAB AT EB 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
*A3	26	#4	STR.	16'-1"	279
A4	26	#4	STR.	15'-11"	276
*B1	52	#5	STR.	11'-2"	606
B2	52	#6	STR.	11'-8"	911
*B3	1	#5	STR.	10'-10"	11
*B4	1	#5	STR.	9'-6"	10
*B5	1	#5	STR.	8'-2"	9
*B6	1	#5	STR.	6'-10"	7
*B7	1	#5	STR.	5'-6"	6
*B8	1	#5	STR.	4'-2"	4
*B9	1	#5	STR.	2'-11"	3
*B10	2	#5	STR.	2'-6"	5
*B11	1	#5	STR.	12'-5"	13
B12	1	#6	STR.	11'-3"	17
B13	1	#6	STR.	9'-11"	15
B14	1	#6	STR.	8'-7"	13
B15	1	#6	STR.	7'-3"	11
B16	1	#6	STR.	5'-11"	9
B17	1	#6	STR.	4'-7"	7
B18	1	#6	STR.	3'-3"	5
B19	2	#6	STR.	2'-6"	8
B20	1	#6	STR.	12'-5"	19
REINFORCING STEEL				LBS.	1291
* EPOXY COATED REINFORCING STEEL				LBS.	953
CLASS AA CONCRETE				C. Y.	14.9
APPROACH SLAB AT EB 2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
*A1	13	#4	STR.	25'-10"	224
A2	13	#4	STR.	25'-10"	224
*B1	52	#5	STR.	11'-2"	606
B2	52	#6	STR.	11'-8"	911
REINFORCING STEEL				LBS.	1135
* EPOXY COATED REINFORCING STEEL				LBS.	830
CLASS AA CONCRETE				C. Y.	13.9



PLAN @ END BENT 1

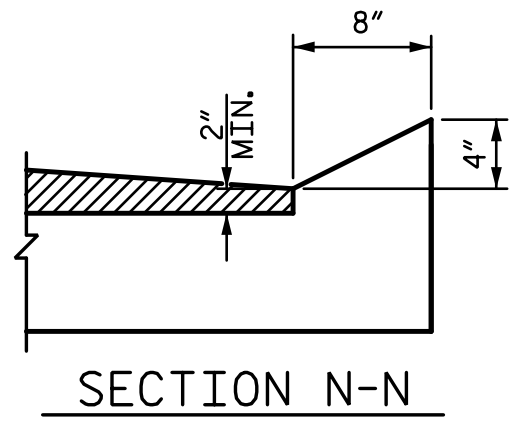


PLAN @ END BENT 2



NOTE: IMMEDIATELY AFTER THE CONSTRUCTION OF THE APPROACH SLAB, THE CONTRACTOR SHALL PROVIDE TEMPORARY BERM AND SLOPE DRAIN. CONTRACTOR SHALL GRADE TO PIPE INLET AND PROVIDE EROSION RESISTANT MATERIAL AS SHOWN. THE EROSION RESISTANT MATERIAL SHALL BE EITHER 1) ASPHALT PLANT MIX, TYPE 1 OR TYPE 2, MIN. 2" DEPTH, 2) EROSION CONTROL MAT, OR 3) CONCRETE, AS DIRECTED BY THE ENGINEER. THE SLOPE DRAIN SHALL CONSIST OF A NON-PERFORATED TEMPORARY DRAINAGE PIPE, 12 INCHES IN DIAMETER.

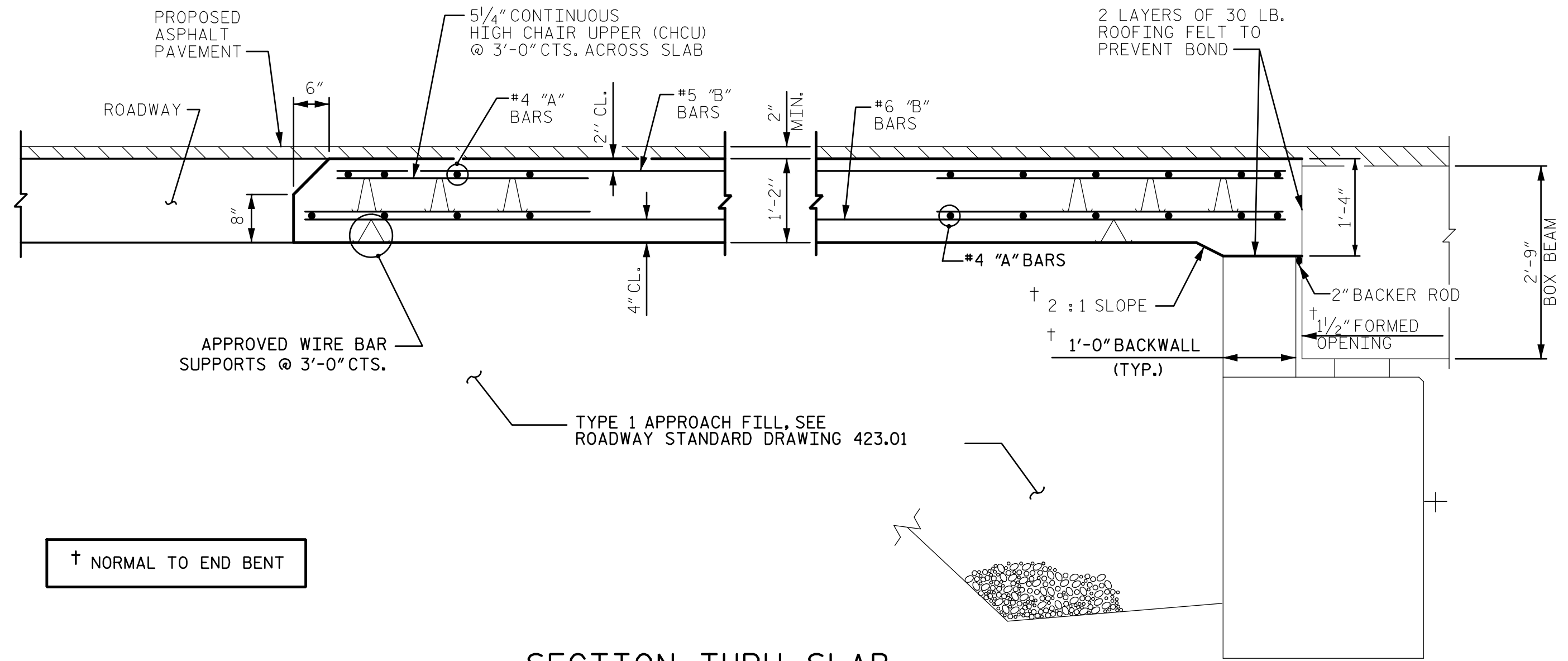
TEMPORARY BERM AND SLOPE DRAIN DETAILS
(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



SECTION N-N CURB DETAILS

SPlice LENGTHS		
BAR SIZE	EPOXY COATED	UNCOATED
#4	1'-11"	1'-7"
#5	2'-5"	2'-0"
#6	3'-7"	2'-5"

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SECTION THRU SLAB

PROJECT NO. **DF18314.2075090**
POLK COUNTY
STATION: **12+15.00 -L-**

10/7/2025
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North Carolina License No. 50773-7-040317-C-28

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE BOX BEAM UNIT (SUB-REGIONAL TIER)					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO.	
S-24	TOTALS 24

DRAWN BY: J. SCACCA DATE: 02/2025
CHECKED BY: T. R. LAWS DATE: 02/2025
DESIGN ENGINEER OF RECORD: T. R. LAWS DATE: 10/2025

STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS	-----	AASHTO (CURRENT)
LIVE LOAD	-----	SEE PLANS
IMPACT ALLOWANCE	-----	SEE AASHTO
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	--	20,000 LBS. PER SQ. IN.
	--	27,000 LBS. PER SQ. IN.
	--	27,000 LBS. PER SQ. IN.
REINFORCING STEEL IN TENSION - GRADE 60	----	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPRESSION	-----	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR	-----	SEE AASHTO
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	----	1,800 LBS. PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	-----	375 LBS. PER SQ. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	-----	30 LBS. PER CU. FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO $1\frac{1}{2}$ " RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A $\frac{1}{4}$ " FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ " RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " \emptyset SHEAR STUDS FOR THE $\frac{3}{4}$ " \emptyset STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " \emptyset STUDS FOR 4 - $\frac{3}{4}$ " \emptyset STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " \emptyset STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " \emptyset STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " \emptyset STUDS FOR 4 - $\frac{3}{4}$ " \emptyset STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST $\frac{5}{16}$ " IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY $\frac{1}{16}$ " OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.